





Developer

on behalf of

Owner













Table of Contents—Appendices

Appendix A-Order of Conditions	5
--------------------------------	---

Appendix B—Transportation Impact Study 2:	2
---	---

	Appendix	C-Certifica	ation of Red	ceipt of P	lans 142
--	----------	-------------	--------------	------------	----------

Certification of Receipt of Plans: Water Department 146

Certification of Receipt of Plans: LEED Specialist 147

Appendix A Order of Conditions



147 Hampshire Street Cambridge, MA 02139 theworks@cambridgema.gov

P: 617 349 4680 F: 617 349 4868

Jennifer Letourneau, Conservation Commission Director

April 27, 2018

David P. Biancavilla, P.E., LEED AP BSC Group 803 Summer Street Boston, MA 02127

Re:

Conservation Commission Paperwork

Order of Conditions #123-282

Mr. Biancavilla:

The attached paperwork must be recorded at the Registry of Deeds and the Recording Information must be returned to my office. Please let me know if you have any questions.

Sincerely,

Jennifer Letourneau, Director Conservation Commission

Cc:

File

DEP-Northeast Region



WPA Form 5 - Order of Conditions

Massachusetts Wetlands Protection Act M.G.L. c. 131, §40

Pr	ovide	ed by	Mas	sDE	Ρ.
12	3-2	82			

MassDEP File #

eDEP Transaction # Cambridge City/Town

Δ	Gener	21	Inform	ation
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Please note: this form has been modified with added space to accommodate the Registry of Deeds Requirements

Cambridge

Conservation Commission

2. This issuance is for (check one):

a. Order of Conditions b. Amended Order of Conditions

3. To: Applicant:

1. From:

Important: When filling out forms on the computer, use only the tab key to move your cursor - do not use the return key.





Tom	Denney	
a. First Name	b. Last Name	
The Hanover Company		
c. Organization		
2 Seaport Lane, 11 th Floor		
d. Mailing Address	·	
Boston	MA	02210
e. City/Town	f. State	g. Zip Code

4. Property Owner (if different from applicant):

Authorization of Agent Docume	nted	
a. First Name	b. Last Name	
c. Organization		
d. Mailing Address		
e. Citv/Town	f. State	a Zin Code

5. F

0. 0.t.j. v 0	1. 5000	g. 21p 0000
Project Location:		
32, 34, 36 CambridgePark Drive	Cambridge	
a. Street Address	b, City/Town	
265D	45, 46, 50, 51	
c. Assessors Map/Plat Number	d, Parcel/Lot Num	ber
Latitude and Longitude, if known:	42d23m38.64s	71d08m37.7s
Editado dira Eongiado, il latowii.	d Latituda	a Laasituda

d. Latitude

wpaform5.doc + rev. 6/16/2015

e. Longitude



WPA Form 5 – Order of Conditions Massachusetts Wetlands Protection Act M.G.L. c. 131, §40

Provided by MassDEP: 123-282
MassDEP File #
eDEP Transaction #
Cambridge
City/Town

A.	Genera	al Informatic	m (con	t.)				
6.	Property rone parce Middlesex		egisti	ry of	Deeds for				
	a. County					b. Certificate Num	iber (if	regist	ered land)
	58587 and	d 58695				205 and 445			
	c. Book					d. Page			
_	Datas	2/7/2018			4/9/2				4/27/2018
7.	Dates:	a. Date Notice of International				e Public Hearing C			c. Date of Issuance
8.	Final Approved Plans and Other Documents (attach additional plan or document references as needed): See NOI File #123-282 a. Plan Title								
	b. Prepared	Ву				c. Signed and Sta	mped	by	
	d. Final Rev	ision Date				e. Scale			
	f. Additional	Plan or Document Titl	е						g. Date
В.	Findin	gs.							
1.	Findings	pursuant to the Ma	assa	chus	etts Wetla	nds Protection .	Act:		
	provided the areas	in this application	and prop	pres osed	ented at th is significa	e public hearing	g, thi:	s Cor iteres	d on the information mmission finds that sts of the Wetlands
a.	Public	c Water Supply	b.		Land Con	taining Shellfisl	n c.		Prevention of lution
d.	☐ Priva	te Water Supply	e.		Fisheries		f.		Protection of dlife Habitat
g.	⊠ Grou	ndwater Supply	h.	\boxtimes	Storm Da	mage Prevention	on i.	\boxtimes	Flood Control
2.	This Com	mission hereby fin	ds th	ie pro	oject, as pro	posed, is: (ched	ck on	e of tl	ne following boxes)
Ар	proved su	bject to:							
a.	standards be perfor General (that the fo	med in accordanc	etla e wi ny of s mo	nds r th the ther s odify	egulations e Notice of special con or differ fro	. This Commiss Intent referenc ditions attached om the plans, sp	sion of ed al d to the pecific	orders bove, his O cation	s that all work shall the following order. To the extent ns, or other



WPA Form 5 - Order of Conditions

Massachusetts Wetlands Protection Act M.G.L. c. 131, §40

Provided by MassDEP: 123-282 MassDEP File#

eDEP Transaction # Cambridge City/Town

B. Findings (cont.)

De	nied because:							
b.	the proposed work cannot be conditioned to meet the performance standards set forth in the wetland regulations. Therefore, work on this project may not go forward unless and until a new Notice of Intent is submitted which provides measures which are adequate to protect the interests of the Act, and a final Order of Conditions is issued. A description of the performance standards which the proposed work cannot meet is attached to this Order.							
c.	the information submitted by the applicant is not sufficient to describe the site, the work, or the effect of the work on the interests identified in the Wetlands Protection Act. Therefore, work on this project may not go forward unless and until a revised Notice of Intent is submitted which provides sufficient information and includes measures which are adequate to protect the Act's interests, and a final Order of Conditions is issued. A description of the specific information which is lacking and why it is necessary is attached to this Order as per 310 CMR 10.05(6)(c).							
3.	Buffer Zone Impacts: Shortest distance between limit of project disturbance and the wetland resource area specified in 310 CMR 10.02(1)(a) a. linear feet							
Inl	and Resource Area Impact	is: Check all tha	at apply below.	(For Approvals O	nly)			
Re	source Area	Proposed Alteration	Permitted Alteration	Proposed Replacement	Permitted Replacement			
4.	Bank	a. linear feet	b. linear feet	c. linear feet	d. linear feet			

				, ,	* /
Res	source Area	Proposed Alteration	Permitted Alteration	Proposed Replacement	Permitted Replacement
4.	Bank	a. linear feet	b. linear feet	c. linear feet	d. linear feet
5.	☐ Bordering				
6.	Vegetated Wetland Land Under	a. square feet	b. square feet	c. square feet	d. square feet
Ŭ.	Waterbodies and Waterways	a. square feet	b. square feet	c. square feet	d. square feet
	,	e. c/y dredged	f. c/y dredged		
7.	■ Bordering Land	57,500	57,500	50,400	50,400
	Subject to Flooding	а. square feet	b. square feet	c. square feet	d. square feet
	Cubic Foot Flood Storage	1,303	1,303	3,122	3,122
	Cubic Feet Flood Storage	e. cubic feet	f. cubic feet	g. cubic feet	h. cubic feet
8.	Isolated Land Subject to Flooding	a. square feet	b. square feet		
	Cubic Feet Flood Storage	c. cubic feet	d. cubic feet	e. cubic feet	f. cubic feet
9.	Riverfront Area	a. total sq. feet	b. total sq. feet		
	Sq ft within 100 ft	c. square feet	d. square feet	e. square feet	f. square feet
	Sq ft between 100- 200 ft	g. square feet	h. square feet	i. square feet	j. square feet



WPA Form 5 – Order of Conditions

Massachusetts Wetlands Protection Act M.G.L. c. 131, §40

Provided by MassDEP: 123-282 MassDEP File #

eDEP Transaction #
Cambridge
City/Town

B. Findings (cont.)

Co	astal Resource Area Impa	cts: Check all tha	at apply below.	(For Approvals 0	Only)
		Proposed Alteration	Permitted Alteration	Proposed Replacement	Permitted Replacement
10.	☐ Designated Port Areas	Indicate size ur	nder Land Unde	r the Ocean, belo	DW WC
11.	Land Under the Ocean	a. square feet	b. square feet		
		c. c/y dredged	d. c/y dredged		
12.	☐ Barrier Beaches	Indicate size ur below	nder Coastal Be	aches and/or Co	astal Dunes
13.	Coastal Beaches	a. square feet	b. square feet	cu yd c. nourishment	cu yd d. nourishment
14.	Coastal Dunes	a. square feet	b. square feet	cu yd c. nourishment	cu yd d. nourishment
15.	Coastal Banks	a. linear feet	b. linear feet		
16.	Rocky Intertidal Shores	a. square feet	b. square feet		
17.	Salt Marshes	a. square feet	b. square feet	c. square feet	d. square feet
18.	Land Under Salt Ponds	a. square feet	b. square feet		
		c. c/y dredged	d. c/y dredged		
19.	☐ Land Containing Shellfish	a. square feet	b. square feet	c. square feet	d. square feet
20.	Fish Runs		or inland Land	nks, Inland Bank Under Waterboo	
0.1	I land Outlands	a. c/y dredged	b. c/y dredged		
21.	Land Subject to Coastal Storm Flowage	a. square feet	b. square feet		
22.	Riverfront Area	a. total sq. feet	b. total sq. feet		
	Sq ft within 100 ft	c. square feet	d. square feet	e. square feet	f. square feet
	Sq ft between 100- 200 ft	g. square feet	h. square feet	i. square feet	j. square feet



WPA Form 5 – Order of Conditions

Massachusetts Wetlands Protection Act M.G.L. c. 131, §40

Provided by MassDEP: 123-282

MassDEP File #

eDEP Transaction #
Cambridge
City/Town

B. Findings (cont.)

* #23. If the project is for the purpose of restoring or enhancing a wetland resource area 2. in addition to the square footage that has been entered in Section B.5.c (BVW) or B.17.c (Salt Marsh) above, 1 please enter the additional amount here. 2.

3.	Restoration/Enhancement *:	
	a. square feet of BVW	b. square feet of salt marsh
4.	Stream Crossing(s):	
	a. number of new stream crossings	b. number of replacement stream crossings

C. General Conditions Under Massachusetts Wetlands Protection Act

The following conditions are only applicable to Approved projects.

- 1. Failure to comply with all conditions stated herein, and with all related statutes and other regulatory measures, shall be deemed cause to revoke or modify this Order.
- 2. The Order does not grant any property rights or any exclusive privileges; it does not authorize any injury to private property or invasion of private rights.
- 3. This Order does not relieve the permittee or any other person of the necessity of complying with all other applicable federal, state, or local statutes, ordinances, bylaws, or regulations.
- 4. The work authorized hereunder shall be completed within three years from the date of this Order unless either of the following apply:
 - a. The work is a maintenance dredging project as provided for in the Act; or
 - b. The time for completion has been extended to a specified date more than three years, but less than five years, from the date of issuance. If this Order is intended to be valid for more than three years, the extension date and the special circumstances warranting the extended time period are set forth as a special condition in this Order.
 - c. If the work is for a Test Project, this Order of Conditions shall be valid for no more than one year.
- 5. This Order may be extended by the issuing authority for one or more periods of up to three years each upon application to the issuing authority at least 30 days prior to the expiration date of the Order. An Order of Conditions for a Test Project may be extended for one additional year only upon written application by the applicant, subject to the provisions of 310 CMR 10.05(11)(f).
- 6. If this Order constitutes an Amended Order of Conditions, this Amended Order of Conditions does not extend the issuance date of the original Final Order of Conditions and the Order will expire on April 27, 2021 unless extended in writing by the Department.
- 7. Any fill used in connection with this project shall be clean fill. Any fill shall contain no trash, refuse, rubbish, or debris, including but not limited to lumber, bricks, plaster, wire, lath, paper, cardboard, pipe, tires, ashes, refrigerators, motor vehicles, or parts of any of the foregoing.



WPA Form 5 – Order of Conditions

Massachusetts Wetlands Protection Act M.G.L. c. 131, §40

Provided by MassDEP: 123-282

MassDEP File #

eDEP Transaction #
Cambridge
City/Town

C. General Conditions Under Massachusetts Wetlands Protection Act

- 8. This Order is not final until all administrative appeal periods from this Order have elapsed, or if such an appeal has been taken, until all proceedings before the Department have been completed.
- 9. No work shall be undertaken until the Order has become final and then has been recorded in the Registry of Deeds or the Land Court for the district in which the land is located, within the chain of title of the affected property. In the case of recorded land, the Final Order shall also be noted in the Registry's Grantor Index under the name of the owner of the land upon which the proposed work is to be done. In the case of the registered land, the Final Order shall also be noted on the Land Court Certificate of Title of the owner of the land upon which the proposed work is done. The recording information shall be submitted to the Conservation Commission on the form at the end of this Order, which form must be stamped by the Registry of Deeds, prior to the commencement of work.
- 10. A sign shall be displayed at the site not less then two square feet or more than three square feet in size bearing the words,

"Massachusetts Department o	f Environmental	Protection" [or	, "MassDEP"
"File Number	123-282	**	

- 11. Where the Department of Environmental Protection is requested to issue a Superseding Order, the Conservation Commission shall be a party to all agency proceedings and hearings before MassDEP.
- 12. Upon completion of the work described herein, the applicant shall submit a Request for Certificate of Compliance (WPA Form 8A) to the Conservation Commission.
- 13. The work shall conform to the plans and special conditions referenced in this order.
- 14. Any change to the plans identified in Condition #13 above shall require the applicant to inquire of the Conservation Commission in writing whether the change is significant enough to require the filing of a new Notice of Intent.
- 15. The Agent or members of the Conservation Commission and the Department of Environmental Protection shall have the right to enter and inspect the area subject to this Order at reasonable hours to evaluate compliance with the conditions stated in this Order, and may require the submittal of any data deemed necessary by the Conservation Commission or Department for that evaluation.
- 16. This Order of Conditions shall apply to any successor in interest or successor in control of the property subject to this Order and to any contractor or other person performing work conditioned by this Order.



WPA Form 5 – Order of Conditions

Massachusetts Wetlands Protection Act M.G.L. c. 131, §40

Provided by MassDEP: 123-282
MassDEP File #

eDEP Transaction # Cambridge City/Town

C. General Conditions Under Massachusetts Wetlands Protection Act (cont.)

- 17. Prior to the start of work, and if the project involves work adjacent to a Bordering Vegetated Wetland, the boundary of the wetland in the vicinity of the proposed work area shall be marked by wooden stakes or flagging. Once in place, the wetland boundary markers shall be maintained until a Certificate of Compliance has been issued by the Conservation Commission.
- 18. All sedimentation barriers shall be maintained in good repair until all disturbed areas have been fully stabilized with vegetation or other means. At no time shall sediments be deposited in a wetland or water body. During construction, the applicant or his/her designee shall inspect the erosion controls on a daily basis and shall remove accumulated sediments as needed. The applicant shall immediately control any erosion problems that occur at the site and shall also immediately notify the Conservation Commission, which reserves the right to require additional erosion and/or damage prevention controls it may deem necessary. Sedimentation barriers shall serve as the limit of work unless another limit of work line has been approved by this Order.

19.	The wo	rk associated with this Order (the "Project")
		is subject to the Massachusetts Stormwater Standards
	(2)	is NOT subject to the Massachusetts Stormwater Standards

If the work is subject to the Stormwater Standards, then the project is subject to the following conditions:

- a) All work, including site preparation, land disturbance, construction and redevelopment, shall be implemented in accordance with the construction period pollution prevention and erosion and sedimentation control plan and, if applicable, the Stormwater Pollution Prevention Plan required by the National Pollution Discharge Elimination System Construction General Permit as required by Stormwater Condition 8. Construction period erosion, sedimentation and pollution control measures and best management practices (BMPs) shall remain in place until the site is fully stabilized.
- b) No stormwater runoff may be discharged to the post-construction stormwater BMPs unless and until a Registered Professional Engineer provides a Certification that:

 i. all construction period BMPs have been removed or will be removed by a date certain specified in the Certification. For any construction period BMPs intended to be converted to post construction operation for stormwater attenuation, recharge, and/or treatment, the conversion is allowed by the MassDEP Stormwater Handbook BMP specifications and that the BMP has been properly cleaned or prepared for post construction operation, including removal of all construction period sediment trapped in inlet and outlet control structures;

 ii. as-built final construction BMP plans are included, signed and stamped by a Registered Professional Engineer, certifying the site is fully stabilized;

 iii. applificit discharges to the stormwater management system have been removed, as per

iii. any illicit discharges to the stormwater management system have been removed, as per the requirements of Stormwater Standard 10;



WPA Form 5 – Order of Conditions

Massachusetts Wetlands Protection Act M.G.L. c. 131, §40

Provided by MassDEP: 123-282 MassDEP File #

eDEP Transaction # Cambridge City/Town

C. General Conditions Under Massachusetts Wetlands Protection Act (cont.)

iv. all post-construction stormwater BMPs are installed in accordance with the plans (including all planting plans) approved by the issuing authority, and have been inspected to ensure that they are not damaged and that they are in proper working condition;

v. any vegetation associated with post-construction BMPs is suitably established to withstand erosion.

- c) The landowner is responsible for BMP maintenance until the issuing authority is notified that another party has legally assumed responsibility for BMP maintenance. Prior to requesting a Certificate of Compliance, or Partial Certificate of Compliance, the responsible party (defined in General Condition 18(e)) shall execute and submit to the issuing authority an Operation and Maintenance Compliance Statement ("O&M Statement) for the Stormwater BMPs identifying the party responsible for implementing the stormwater BMP Operation and Maintenance Plan ("O&M Ptan") and certifying the following:
 - i.) the O&M Plan is complete and will be implemented upon receipt of the Certificate of Compliance, and
 - ii.) the future responsible parties shall be notified in writing of their ongoing legal responsibility to operate and maintain the stormwater management BMPs and implement the Stormwater Pollution Prevention Plan.
- d) Post-construction pollution prevention and source control shall be implemented in accordance with the long-term pollution prevention plan section of the approved Stormwater Report and, if applicable, the Stormwater Pollution Prevention Plan required by the National Pollution Discharge Elimination System Multi-Sector General Permit.
- e) Unless and until another party accepts responsibility, the landowner, or owner of any drainage easement, assumes responsibility for maintaining each BMP. To overcome this presumption, the landowner of the property must submit to the issuing authority a legally binding agreement of record, acceptable to the issuing authority, evidencing that another entity has accepted responsibility for maintaining the BMP, and that the proposed responsible party shall be treated as a permittee for purposes of implementing the requirements of Conditions 18(f) through 18(k) with respect to that BMP. Any failure of the proposed responsible party to implement the requirements of Conditions 18(f) through 18(k) with respect to that BMP shall be a violation of the Order of Conditions or Certificate of Compliance. In the case of stormwater BMPs that are serving more than one lot, the legally binding agreement shall also identify the lots that will be serviced by the stormwater BMPs. A plan and easement deed that grants the responsible party access to perform the required operation and maintenance must be submitted along with the legally binding agreement.
- f) The responsible party shall operate and maintain all stormwater BMPs in accordance with the design plans, the O&M Plan, and the requirements of the Massachusetts Stormwater Handbook.



WPA Form 5 – Order of Conditions

Massachusetts Wetlands Protection Act M.G.L. c. 131, §40

Provided by MassDEP: 123-282
MassDEP File #

eDEP Transaction #
Cambridge
City/Town

C. General Conditions Under Massachusetts Wetlands Protection Act (cont.)

- g) The responsible party shall:
 - 1. Maintain an operation and maintenance log for the last three (3) consecutive calendar years of inspections, repairs, maintenance and/or replacement of the stormwater management system or any part thereof, and disposal (for disposal the log shall indicate the type of material and the disposal location);
 - 2. Make the maintenance log available to MassDEP and the Conservation Commission ("Commission") upon request; and
 - 3. Allow members and agents of the MassDEP and the Commission to enter and inspect the site to evaluate and ensure that the responsible party is in compliance with the requirements for each BMP established in the O&M Plan approved by the issuing authority.
- h) All sediment or other contaminants removed from stormwater BMPs shall be disposed of in accordance with all applicable federal, state, and local laws and regulations.
- i) Illicit discharges to the stormwater management system as defined in 310 CMR 10.04 are prohibited.
- j) The stormwater management system approved in the Order of Conditions shall not be changed without the prior written approval of the issuing authority.
- k) Areas designated as qualifying pervious areas for the purpose of the Low Impact Site Design Credit (as defined in the MassDEP Stormwater Handbook, Volume 3, Chapter 1, Low Impact Development Site Design Credits) shall not be altered without the prior written approval of the issuing authority.
- Access for maintenance, repair, and/or replacement of BMPs shall not be withheld.
 Any fencing constructed around stormwater BMPs shall include access gates and shall be at least six inches above grade to allow for wildlife passage.

Special Conditions (if you need more space for additional conditions, please attach a text

document): See Attachment	, ,

20. For Test Projects subject to 310 CMR 10.05(11), the applicant shall also implement the monitoring plan and the restoration plan submitted with the Notice of Intent. If the conservation commission or Department determines that the Test Project threatens the public health, safety or the environment, the applicant shall implement the removal plan submitted with the Notice of Intent or modify the project as directed by the conservation commission or the Department.



WPA Form 5 - Order of Conditions

Massachusetts Wetlands Protection Act M.G.L. c. 131, §40

Provided by MassDEP: 123-282 MassDEP File # eDEP Transaction #

Cambridge City/Town

D.	Findings	Under	Municipal	Wetlands	Bylaw	or Ordinance

1.	ls a	a municipal wetlands bylaw or ordinance applicable? 🔲 Yes 🛮 🖾 No		
2.	Th	hereby finds (check one that applies): Conservation Commission that the proposed work cannot be conditioned to meet the standards set forth in a		
	u.	municipal ordinance or byław, specifically:		
		1. Municipal Ordinance or Bylaw 2. Citation		
		Therefore, work on this project may not go forward unless and until a revised Notice of Intent is submitted which provides measures which are adequate to meet these standards, and a final Order of Conditions is issued.		
	b.			
		1. Municipal Ordinance or Bylaw 2. Citation		
3.	100	e Commission orders that all work shall be performed in accordance with the following nditions and with the Notice of Intent referenced above. To the extent that the following nditions modify or differ from the plans, specifications, or other proposals submitted with Notice of Intent, the conditions shall control.		
	Th	e special conditions relating to municipal ordinance or bylaw are as follows (if you need ore space for additional conditions, attach a text document):		



WPA Form 5 - Order of Conditions

Massachusetts Wetlands Protection Act M.G.L. c. 131, §40

Provided by MassDEP: 93-98

MassDEP File #

eDEP Transaction #

E. Signatures

This Order is valid for three years, unless otherwise specified as a special condition pursuant to General Conditions #4, from the date of issuance.

Please indicate the number of members who will sign this form.

This Order must be signed by a majority of the Conservation Commission.

2. Number of Signers

The Order must be mailed by certified mail (return receipt requested) or hand delivered to the applicant. A copy also must be mailed or hand delivered at the same time to the appropriate Department of Environmental Protection Regional Office, if not filing electronically, and the property owner, if different from applicant.

Signatures:

by hand delivery on

by certified mail, return receipt requested, on

Date

F. Appeals

The applicant, the owner, any person aggrieved by this Order, any owner of land abutting the land subject to this Order, or any ten residents of the city or town in which such land is located. are hereby notified of their right to request the appropriate MassDEP Regional Office to issue a Superseding Order of Conditions. The request must be made by certified mail or hand delivery to the Department, with the appropriate filing fee and a completed Request for Departmental Action Fee Transmittal Form, as provided in 310 CMR 10.03(7) within ten business days from the date of issuance of this Order. A copy of the request shall at the same time be sent by certified mail or hand delivery to the Conservation Commission and to the applicant, if he/she is not the appellant.

Any appellants seeking to appeal the Department's Superseding Order associated with this appeal will be required to demonstrate prior participation in the review of this project. Previous participation in the permit proceeding means the submission of written information to the Conservation Commission prior to the close of the public hearing, requesting a Superseding Order, or providing written information to the Department prior to issuance of a Superseding Order.

The request shall state clearly and concisely the objections to the Order which is being appealed and how the Order does not contribute to the protection of the interests identified in the Massachusetts Wetlands Protection Act (M.G.L. c. 131, § 40), and is inconsistent with the wetlands regulations (310 CMR 10.00). To the extent that the Order is based on a municipal ordinance or bylaw, and not on the Massachusetts Wetlands Protection Act or regulations, the Department has no appellate jurisdiction.



WPA Form 5 – Order of Conditions

Massachusetts Wetlands Protection Act M.G.L. c. 131, §40

Provided by MassDEP: 123-282
MassDEP File #

eDEP Transaction #
Cambridge
City/Town

G. Recording Information

Cambridge

Prior to commencement of work, this Order of Conditions must be recorded in the Registry of Deeds or the Land Court for the district in which the land is located, within the chain of title of the affected property. In the case of recorded land, the Final Order shall also be noted in the Registry's Grantor Index under the name of the owner of the land subject to the Order. In the case of registered land, this Order shall also be noted on the Land Court Certificate of Title of the owner of the land subject to the Order of Conditions. The recording information on this page shall be submitted to the Conservation Commission listed below.

Conservation Commission		
Detach on dotted line, have stamped by the F	Registry of Deeds and s	submit to the Conservation
Commission.		
То:		
Cambridge		
Conservation Commission		
Please be advised that the Order of Condition	ons for the Project at:	
32, 34, 36 CambridgePark Drive	123-282	
Project Location	MassDEP File Nu	mber
Has been recorded at the Registry of Deeds	of:	
County	Book	Page
for: Property Owner		
and has been noted in the chain of title of the	e affected property in:	
Book	Page	
In accordance with the Order of Conditions i	ssued on:	
April 27, 2018		
If recorded land, the instrument number iden	itifying this transaction	is:
Instrument Number		
If registered land, the document number iden	ntifying this transaction	is:
Document Number		
Signature of Applicant		

DEP File #123-282 32, 34, 36 CambridgePark Drive (aka 50 CambridgePark Drive) Cambridge, Massachusetts

Documents and Plans:

The complete file is available for review in the Cambridge Conservation Commission office at 147 Hampshire Street, Cambridge, MA 02139.

- Notice of Intent dated February 7, 2018
- Review Memorandum dated March 11, 2018 from Cambridge DPW
- Revised Stormceptor Calculations dated April 11, 2018

Special Conditions:

- 18. Work shall conform to the Notice of Intent under the Massachusetts Wetlands Protection Act, M.G.L. ch. 131, sec. 40, submitted to the Cambridge Conservation Commission on **February 7, 2018** and the additional information and modifications outlined in the supplemental documents and plans provided by the applicant. Specifically, the proposed work shall conform to the most recent revisions to the Notice of Intent document and plans, received by the Commission as stated above.
- 19. Any further proposed or executed changes in the plans approved under this Order shall require the applicant to seek an amended Order of Conditions or to file a new Notice of Intent, or to inquire of the Cambridge Conservation Commission in writing whether the change or changes is/are substantial enough to require a new filing. Any errors in the plans or information by the applicant shall be considered changes and the above procedures shall be followed.
- 20. Prior to any work on the site, the applicant shall record this Order of Conditions at the Registry of Deeds pursuant to Condition 8. Failure to do so shall be deemed cause to revoke this Order.
- 21. The applicant shall provide to the Conservation Commission copies of all other permits, variances, licenses or determinations which may be necessary for this project by other local, state and federal agencies at least 2 weeks prior to commencement of work.
- 22. This Order of Conditions shall be included in all construction contracts and subcontracts dealing with the work proposed and shall supersede all conflicting contract requirements that are less protective of Wetland Resource Areas.
- 23. A copy of the final construction documents will be provided to the Conservation Commission at least 2 weeks prior to commencement of work.
- 24. As soon as possible, final means and methods for construction activities within the wetland resource areas will be provided to the Conservation Commission.

- 25. The applicant and its contractor shall keep at least one copy of the approved Order of Conditions at the project site during construction.
- 26. The applicant or its agent shall specify to the Commission, prior to commencement of activity on the site, the name and telephone number of the person(s) designated by the applicant to be responsible for compliance with the conditions of this Order on the site and his/her alternate.
- 27. The applicant or its agent shall notify the Commission at least 72 hours in advance of completing the installation of erosion controls, sediment controls and tree protection. A site inspection with the Director is required prior to commencement of construction.
- 28. The applicant shall provide to the Conservation Commission copies of project inspectional reports during construction including but not limited to maintenance and operation and vegetation monitoring.
- 29. The members and agents of the Conservation Commission shall have the right to enter the site to verify compliance with this Order and to require the submittal of additional data deemed necessary by the Commission for that verification. The Commission understands that construction-site safety procedures must be followed during site visits.
- 30. During project construction, the applicant or its contractors shall provide and maintain free and safe passage by pedestrians and bicyclists along the roads or walkways adjacent to the site.
- 31. If some unexpected or unforeseen event occurs, that needs to be addressed, all work shall stop until the event can be brought to the attention of the Director of the Commission and a decision made by the Director as to whether it needs to be brought before the Commission.
- 32. If a workday commences with heavy rain, no work shall take place in the buffer zone or resource area that day. If heavy rain commences after start of work, all work shall cease in the buffer zone or resource area for that day, and appropriate sedimentation and erosion control shall be in place, to prevent any sedimentation to the river and other resource areas.
- 33. Trucks entering and leaving the site shall have their loads completely covered in compliance with M.G.L. Chapter 85 section 36. The applicant shall also instruct all drivers on site that vehicles shall not idle for longer than 5 minutes in compliance with M.G.L. Chapter 90 section 16A.
- 34. Construction mitigation measures must be inspected regularly and maintained for the duration of the construction project.

- 35. A final landscape restoration and maintenance plan must be submitted and approved prior to performing these activities.
- 36. A Certificate of Compliance may be issued only after the construction is complete, the site has been restored and the landscape has been restored and maintained for 3 years.

Appendix B Transportation Impact Study

50 Cambridgepark Drive Development

Cambridge, Massachusetts

PREPARED FOR

Hanover RS Limited Partnership c/o The Hanover Company 2 Seaport Lane, 11th Floor Boston, MA 02210

PREPARED BY



99 High Street Boston, MA 02110 617.728.7777

May 29, 2018



UNDER THE DIRECTION OF

Ryan White, P.E.

Massachusetts Registration No. 52666



Table of Contents

Tab	le of Co	ntents	1
List	of Table	es	3
	_	res	
		n & Project Overview	
		verview	
T.	IS Study	r Area	3
Р	lanning	Board Criteria Summary	3
Trar	•	ion Impact Study	
1	Inve	ntory of Existing Conditions	S
	1.a	Roadways	
	1.b	Intersections	S
	1.c	Parking	10
	1.d	Transit Services	10
	Publ	ic Transit Services	10
	Priva	ite Transit Services	11
	1.e	Land Use	11
2	Data	Collection	11
	2.a	ATR Counts	11
	2.b	Pedestrian and Bicycle Counts	13
	2.c	Intersection Turning Movement Counts and Queues	14
	2.d	Crash Analysis	17
	2.e	Public Transit	18
	2.f	Parking	18
3	Proje	ect Traffic	19
	3.a	Mode Share and Vehicle Occupancy Rate	19
	3.b	Trip Generation and Trip Credit for Existing Use on Site	19
	3.c	Trip Distribution and Assignment	22
	3.d	Service and Loading	23
4	Back	ground Traffic	24
5	Traff	ic Analysis	24
	5.a	2018 Existing Condition	



5.b	2018 Build Condition	24
5.c	2023 Future Condition	25
6 Ve	hicle Capacity Analysis	25
6.a	Capacity Analysis	25
7 Qı	ueue Analysis	33
8 Re	sidential Street Volume Analysis	35
9 Pa	rking Analysis	38
9.a	Vehicle Parking	38
9.a	Bicycle Parking	40
10 Tr	ansit Analysis	40
10.a	Existing Transit System Capacity – STEP 1	41
10.k	Existing Transit System Ridership and Utilization – Steps 2 & 3	43
10.0	Development of Transit Project Trips – Step 4	43
10.0	Build Transit System Utilization – Step 5	44
10.€	Development of Future Transit Trips – Step 6	45
10.f	Compile and Assign Area Background Project Transit Trips – Step 7	45
10.8	Future Transit System Utilization – Step 8	46
11 Pe	destrian Analysis	47
12 Bi	cycle Analysis	49
12.a	Conflicting Movements	49
13 Tr	ansportation Demand Management	51
14 Tr	ansportation Mitigation	52
Planning	Board Special Permit Criteria	53
Criterio	on A – Project Vehicle Trip Generation	53
Criterio	on B – Vehicle LOS	53
Criterio	on C – Traffic on Residential Streets	55
Criterio	on D – Lane Queue	56
Criterio	on E – Pedestrian and Bicycle Facilities	57
Crite	eria 1: Pedestrian Delay	57
Crite	eria 2 & 3: Safe Pedestrian and Bicycle Facilities	59

ii



List of Tables

Α	Existing Site Conditions and Uses	
В	Proposed Development Program	3
2.a.1	Existing Traffic Volume Summary (November 2018)	12
2.a.2	Existing Average Daily Traffic Summary (November 2018)	13
2.b.1	Existing 12-hour Pedestrian and Bicycle Volumes (November 20	018) 14
2.c.1	Signalized Intersection Queue Observations (# of Cars)	16
2.d.1	MassDOT Crash Analysis (January 2013 – December 2015)	17
2.e.1	MBTA Services	18
3.a.1	Mode Share	19
3.b.1	Vehicle Trip Rates	20
3.b.2	Project residential Use Generated Trips (Before Existing Use Cre	edit).20
3.b.3	Project Retial/Restaurant Use Generated Trips (Before Existing Credit)	
3.b.4	Total Project Generated Trips (Before Existing Use Credit)	
3.b.5	Vehicle Trip Credit	
3.b.6	Net-New Project Generated Vehicle Trips	
3.c.1	Summary of Project Vehicle Trip Distribution	
3.c.2	Summary of Existing Site Credit Vehicle Trip Distribution	
6.a.1	Signalized Intersection Level of Service Results –	
	Morning Peak Hour	27
6.a.2	Signalized Intersection Level of Service Results –	
	Evening Peak Hour	29
6.a.3	Unsignalized Intersection Level of Service Results –	
	Morning Peak Hour	31
6.a.4	Unsignalized Intersection Level of Service Results –	
	Evening Peak Hour	31
6.a.5	Rotary Level of Service Results –	
	Morning Peak Hour	32
6.a.6	Rotary Level of Service Results –	
	Evening Peak Hour	32
7.a.1	Signalized Intersection Queue Analysis – Morning Peak Hour	33
7.a.2	Signalized Intersection Queue Analysis – Evening Peak Hour	34
7.a.3	Cambridgepark Drive at Steel Place Queue Analysis – Evening F Hour	
8.a.1	Traffic on Study Area Roadway - Morning Peak Hour	36
8.a.2	Traffic on Study Area Roadways – Evening Peak Hour	
9.a.1	Area Residential Building Parking Rates	38
	5 5	



9.a.2	Estimated Average Weekday Project Parking Occupancy	39
9.b.1	Bicycle Parking	
10.a.1	System Peak Hour Capacity (Per MBTA Data)	42
10.a.2	Future System Peak Hour Capacity (Per MBTA Data)	
10.b.1	Existing Transit Service Utilization (Per MBTA Data)	
10.c.1	Transit Trip Distribution	44
10.c.2	Project-Generated Transit Trips by Line	44
10.d.1	Build Condition Transit Service Utilization (Per MBTA Data)	
10.e.1	2023 Future Growth and Background Project Transit Trips	45
10.f.1	Background Project Transit Trips	46
10.g.1	2023 Future Condition Transit Service Utilization	
11.a.1	Signalized Intersection – Pedestrian LOS Summary	48
11.a.2	Unsignalized Intersection – Pedestrian LOS Summary	48
12.a.1	Conflicting Bicycle/Vehicle Movement at Study Intersection	49
14.a.1	Exceedance Mitigation Summary	52
A-1	Project Vehicle Trip Generation	53
B-1	Criterion – Vehicular Level of Service	
B-2	Vehicular Level of Service	54
C-1	Criterion – Traffic on Residential Streets	55
C-2	Traffic on Residential Streets	55
D-1	Criterion – Vehicular Queues at Signalized Intersections	56
D-2	Length of Vehicular Queues at Signalized Intersections	57
E-1	Criterion – PLOS Indicators	58
E-2	Study Area Intersections PLOS Summary	58
E-3	Pedestrian and Bicvcle Facilities	59



List of Figures

A	Site Location Map
В	Project Site
C	Existing Conditions Site Plan
D	Proposed Site Plan
E	TIS Study Area Intersections
F.1	Proposed Vehicular Parking Plans – Ground Floor Floor
F.2	Proposed Vehicle Parking Plans – Second FloorFloor
G.1	Proposed Long-Term Bike Parking
G.2	Proposed Long-Term Bike Parking
G.3	Proposed Short-Term Bike Parking
1.b.1	Cambridgepark Drive at 100 Cambridgepark Drive and Site Driveways
1.b.2	Cambridgepark Drive at Steel Place
1.b.3	Cambridgepark Drive at Alewife Brook Parkway
1.b.4	Cambridgepark Drive at Rindge Avenue
1.b.5	Steel Place at Alewife Station Access Road (Route 2 Connector)
1.b.6	Fresh Pond Rotary
1.b.7	Alewife Brook Parkway at Route 2/16
1.d.1	Public Transit
1.d.2	Private Transit Services
1.d.3	Bike and Car Sharing Services
1.e.1	Current Land Use
2.a.1	Cambridgepark Drive, West of Steel Place – Daily ATR Summary
2.a.2	Cambridgepark Drive, East of Steel Place – Daily ATR Summary
2.a.3	Steel Place, North of Cambridgepark Drive – Daily ATR Summary
2.a.4	Alewife Brook Parkway, North of Cambridgepark Drive – Daily ATR Summary
2.a.5	Cambridgepark Drive, West of Steel Place – PM Peak Period Summary
2.a.6	Cambridgepark Drive, East of Steel Place – PM Peak Period Summary
2.a.7	Steel Place, North of Cambridgepark Drive – PM Peak Period Summary
2.a.8	Alewife Brook Parkway, North of Cambridgepark Drive – PM Peak Period Summary
2.c.1	2018 Existing Condition Morning Peak Hour Vehicle Volumes
2.c.2	2018 Existing Condition Evening Peak Hour Vehicle Volumes
2.c.3	2018 Existing Condition Bicycle Volumes
2.c.4	2018 Existing Condition Pedestrian Volumes
3.c.1	Project Trip Distribution
3.c.2	Existing Site Trip Distribution
3.c.3	Total Project Generated Trips Morning Peak Hour Vehicle Volumes.
3.c.4	Total Project Generated Trips Evening Peak Hour Vehicle Volumes



3.c.5 3.c.6 3.d.1 3.d.2	Net New Project Generated Trips Morning Peak Hour Vehicle Volumes Net New Project Generated Trips Evening Peak Hour Vehicle Volumes Service and Loading Loading Dock Truck Turns
4.c.1	2018 Build Condition Morning Peak Hour Vehicle Volumes
4.c.2	2018 Build Condition Evening Peak Hour Vehicle Volumes
5.c.1	2023 Future Condition Morning Peak Hour Vehicle Volumes
5.c.2	2023 Future Condition Evening Peak Hour Vehicle Volumes
5.c.3	Cumulative Area Developments Impact Evening Peak Hour Vehicle Volumes
6.a.1	AM Peak Vehicle Level of Service
6.a.2	PM Peak Vehicle Level of Service
6.b.1	AM Peak Net Change in Vehicle Delay
6.b.2	PM Peak Net Change in Vehicle Delay
11.a.1	AM Peak Pedestrian Level of Service
11.a.2	PM Peak Pedestrian Level of Service



Introduction & Project Overview

On behalf of Hanover RS Limited Partnership (the Owner), VHB, Inc. has conducted a Transportation Impact Study (TIS) for the proposed 50 Cambridgepark Drive residential development (the Project Site) for up to 299 residential units and approximately 7,000 square feet of ground floor retail/restaurant space. The Project will be supported by an internal parking structure with approximately 187 vehicle parking spaces and approximately 315 long-term interior bicycle spaces along with 37 exterior short-term bicycle parking spaces (the Proposed Project).

The TIS responds to the scope dated March 2, 2018 defined by the City of Cambridge Traffic, Parking and Transportation (TP&T) Department in response to VHB's Request for Scoping dated January 25, 2018. Copies of the City's scoping letter and VHB's Request for Scoping are included in the Appendix. The TIS has been prepared in conformance with the current City of Cambridge Guidelines for Transportation Impact Studies, as required under the Article 19 Special Permit Project Review. This document is comprised of three sections, as follows:

- Introduction and Project Overview describing the framework in which the transportation component of the Project was evaluated;
- Transportation Impact Study (TIS) presenting the technical information and analysis results as required under the guidelines; and,
- Planning Board Special Permit Criteria summarizing the evaluation of the proposed
 Project as defined under the guidelines.

The required TIS Summary Sheets and Planning Board Criteria Performance Summary are included. Supplementary data and analysis worksheets are provided on an accompanying CD. Electronic files for Automatic Traffic Recorder (ATR) counts, Turning Movement Counts (TMC), and Synchro analyses are included on an accompanying CD.

Project Overview

The Proposed Project will consider the development of up to 299 residential units and approximately 7,000 square feet of ground floor retail/restaurant that will be supported by approximately 187 new parking spaces contained within the building, as well as approximately 315 long term bicycle parking spaces and 38 short term bicycle parking spaces, in accordance with the City's Bicycle Parking Guidelines.

The following figures illustrate details of the Proposed Project program.

Figure A presents a regional context site location map.



- Figure B presents a neighborhood context site location map.
- Figure C presents the existing conditions of the proposed site.
- Figure D presents the proposed site plan.
- Figure E presents the TIS study area.
- Figure F.1 F.2 present the proposed on-site parking layout
- Figure G.1 G.3 present the proposed bicycle parking layout

As shown in Figures A and B, the Project consists of an approximately 79,325 square foot site on Cambridgepark Drive in Cambridge, Massachusetts. This site will contain a new residential building with street level retail/restaurant use and parking.

The Proposed Project consists of up to 299 residential units within a single, eight-story building. The Proposed Project will include affordable units per City of Cambridge requirements and will also have common lobby and amenity spaces for its residents on the first two floors. The ground floor will also house main MEP rooms, meters, bike parking, trash, loading dock and similar back of house spaces, but there will also be additional mechanical allocation at the roof of the buildings and MEP closets on each floor. Above the first level, the units will generally stack vertically, but it is expected that there may be some unique units in places that may take advantage of the views, corners or the façade composition. The vehicular parking is located within the building on the first and second floors.

As shown in Figure C, the site currently contains three office/research buildings totaling approximately 39,000 square feet that will be demolished as part of the project. The surface parking lot currently supporting the office building will also be demolished.

TABLE A EXISTING SITE CONDITIONS AND USES

Existing Building	Size / Quantity	
Square Footage	39,000 SF	
Land Use	Office/Research	
# of Parking Spaces	68 spaces ¹	

¹Source: City's 1990 parking inventory

Figure D presents the proposed 50 Cambridgepark Drive Development site plan. As noted above, the site will include up to 299 residential units and 7,000 square feet of retail/restaurant space. As part of the Project, the current 100 Cambridgepark Drive driveway will be reconstructed and serve as a shared driveway for the 50 Cambridgepark Drive Development as well as 88 Cambridgepark Drive, 100 Cambridgepark Drive and 130 Cambridgepark Drive. The shared driveway will provide sidewalks, streetscape and on-street parking.

It is currently envisioned that 187 parking spaces will be provided for residential parking. In addition, 9 on-street spaces will be provided along the building frontage of the new shared driveway.

The Proposed Project program is summarized in Table B below.



TABLE B PROPOSED DEVELOPMENT PROGRAM

Project Component	Size / Quantity
Residential	299 units (309,000 GSF)
Retail/Restaurant	7,000 square feet
Vehicle Parking	187 spaces within the building (0.63 spaces/unit); 9 spaces along driveway for short-term use
Bicycle Parking	315 long term spaces, and
	38 short-term spaces

TIS Study Area

The TIS study area for the Proposed Project, as defined by the City of Cambridge, is shown in Figure E. The study intersections include the following:

- 1. Cambridgepark Drive/100 Cambridgepark Drive Driveway
- 2. Cambridgepark Drive/Site West (outbound) Driveway
- 3. Cambridgepark Drive/Site East (inbound) Driveway
- 4. Cambridgepark Drive/Steel Place (signalized)
- 5. Cambridgepark Drive/Alewife Brook Parkway (signalized)
- 6. Alewife Brook Parkway/Rindge Avenue (signalized)
- 7. Steel Place/Alewife Station Access Road (Route 2 Connector)
- 8. Fresh Pond Rotary
- 9. Alewife Brook Parkway at Route 2/16 (signalized)

Planning Board Criteria Summary

Based on the TIS analysis, the Project has been evaluated within the context of the Planning Board Criteria to determine if the Project has any potential adverse transportation impacts. Exceeding one or more of the Criteria is indicative of a potentially adverse impact on the City's transportation network. However, the Planning Board will consider mitigation efforts, their anticipated effectiveness, and other information that identifies a reduction in adverse transportation impacts.

The Planning Board Criteria consider the Project's vehicular trip generation, impact to intersection level of service and queuing, as well as increase of volume on residential streets. In addition, pedestrian and bicycle conditions are considered. A discussion of the Criteria set forth by the Planning Board is presented in the final section of the TIS, and the Planning Board Criteria Performance Summary is presented below.

The Project has an estimated 14 exceedances out of 143 data entries. All exceedances are due to existing pedestrian crossing and infrastructure conditions.

CITY OF CAMBRIDGE

Special Permit – Transportation Impact Study (TIS) Planning Board Criteria Performance Summary 50 Cambridgepark Drive Development

Dlanning	Roard	Dormit	Number:	TBD	
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PROJECT

Project Name: 50 Cambridgepark Drive Development

Project Address: 50 Cambridgepark Drive

Cambridge, MA 02138

Owner/Developer Name: Hanover RS Limited Partnership

Contact Person: David S. Hall

Contact Address: c/o The Hanover Company 2 Seaport Lane, 11th Floor

Boston, MA 02210 dhall@hanoverco.com

Contact Phone Number: (857) 400-0681

SIZE

ITE sq. ft.: 309,000 GSF – 299 residential units

Land Use Type: Residential ITE sq. ft.: 7,000 SF

Land Use Type: Retail/Restaurant

PARKING

Existing Parking Spaces: 68 Use: Office/Research New Parking Spaces: 187 Use: Residential

Net New Parking Spaces: +119

TRIP GENERATION*:

	Daily	Morning Peak Hour	Evening Peak Hour
Total Trips	2,343	_	_
SOV	578	61	63
HOV	24	2	2
Transit	926	101	105
Bike	122	13	13
Walk	634	57	54
Other	116	12	13

^{*} Does not include trips eliminated by elimination of existing site use

MODE SPLIT (Person Trips)

	Residential	Retail/Restaurant
SOV	28%	18%
HOV	2%	2%
Transit	51%	20%
Bike	5%	5%
Walk	8%	52%
Other	6%	3%

TRANSPORTATION CONSULTANT

Company Name: VHB

4

Contact Name: R. David Black Contact Phone Number: 617-607-2906

Date of Building Permit Approval:

Planning	Board	Permit	Number:	TBD	
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Planning Board Criteria

Total Data Entries = 143

Total Number of Criteria Exceedances = 14

Criteria A – Project Vehicle Trip Generation

Time Period	Criteria (trips)	Build*	Exceeds Criteria?
Weekday Daily	2,000	602	No
Weekday Moring Peak Hour	240	63	No
Weekday Evening Peak Hour	240	65	No

^{*} Does not include trips eliminated by elimination of existing site use

Criteria B - Vehicular LOS

Morning Peak Hour Ev						Evening	Peak Hour	
Intersection	Existing Condition	Build Condition	Traffic Increase	Exceeds Criterion?	Existing Condition	Build Condition	Traffic Increase	Exceeds Criterion?
Cambridgepark Drive/100 Cambridgepark Drive Driveway	С	С	9%	No	В	В	11%	No
Cambridgepark Drive/Site West (outbound) Driveway	А	-	9%	No	В	-	9%	No
Cambridgepark Drive/Site East (inbound) Driveway	А	-	6%	No	А	-	9%	No
Cambridgepark Drive/Steel Place	С	С	4%	No	С	С	3%	No
Cambridgepark Drive/Alewife Brook Parkway	E	E	1%	No	F	F	1%	No
Alewife Brook Parkway/Rindge Avenue	F	F	1%	No	F	F	1%	No
Steel Place/Alewife Station Access Road (Route 2 Connector)	F	F	0%	No	F	F	0%	No
Fresh Pond Rotary	F	F	1%	No	F	F	1%	No
Alewife Brook Parkway at Route 2/16 – Signal A	В	В	0%	No	В	В	0%	No
Alewife Brook Parkway at Route 2/16 – Signal B	E	E	0%	No	F	F	0%	No
Alewife Brook Parkway at Route 2/16 – Signal C	С	С	3%	No	В	В	0%	No
Alewife Brook Parkway at Route 2/16 – Signal D	В	В	0%	No	А	А	1%	No

5

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Criteria C – Traffic on Residential Streets

			Morning Peak Hour			Evening Peak Hour		
Roadway	Segment	Amount of Residential	Existing ¹	Increase ²	Exceeds Criteria?	Existing ¹	Increase ²	Exceeds Criteria?
Cambridgepark Drive	west of 100 Cambridgepark Dr	> 1/3 but <1/2	621	0	No	425	0	No
	between 100 Cambridgepark Dr and Site West Driveway	1/3 or less	736	63	No	574	65	No
	between Site West Driveway and Site East Driveway	1/3 or less	736	63	No	588	51	No
	between Site East Driveway and Steel Pl	1/3 or less	754	45	No	587	50	No
	between Steel PI and Alewife Brook Parkway	1/3 or less	979	42	No	1261	46	No
Steel Place	between Cambridgepark Dr and Alewife Station Access Rd	1/3 or less	727	3	No	799	2	No
	north of Alewife Station Access Rd	1/3 or less	1099	-7	No	922	2	No
Rindge Avenue	west of Cambridgepark Dr	1/2 or more	948	1	No	813	6	No
Concord Avenue	west of Fresh Pond Rotary	1/3 or less	1765	13	No	1325	14	No
	east of Fresh Pond Rotary	1/3 or less	3550	18	No	3010	19	No
Alewife Brook Parkway	between Fresh Pond Rotary and Rindge Ave	1/3 or less	3200	31	No	3091	33	No
	between Rindge Ave and Cambridgepark Dr	1/3 or less	3738	32	No	3503	39	No
	Between Cambridgepark Dr and Route 2/16 Interchange	1/3 or less	3411	10	No	3180	7	No
	north of Route 2/16 Interchange	1/3 or less	2344	12	No	2578	12	No
Route 2	west of Route 2/16 Interchange	1/3 or less	4251	8	No	4558	-5	No
Alewife Station Access Road	between Route 2/16 Interchange and Steel Place	1/3 or less	285	10	No	801	0	No

¹ Where driveways/on-street parking created a segment inflow/outflow volume imbalance, an average was calculated per direction and added

² Net new project trips after trip credits are applied

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Criteria D – Lane Queue (for signalized intersections)

		Morr	ning Peak	Hour	Even	ing Peak	Hour
Intersection	Lane	2018 Existing	2018 Build	Exceeds Criteria?	2018 Existing	2018 Build	Exceeds Criteria?
	Steel Place NB L/T/R	1	1	No	1	1	No
	Steel Place SB L	4	4	No	7	7	No
Cambridgepark	Steel Place SB L/T/R	1	1	No	7	7	No
Drive/Steel Place	Cambridgepark Drive EB L/T/R	4	5	No	8	8	No
	Cambridgepark Drive WB L/T	10	11	No	4	5	No
	Cambridgepark Drive WB R	0	0	No	0	0	No
	Alewife Brook Parkway NB L	4*	5*	No	4*	4*	No
Cambridgepark	Alewife Brook Parkway NB T	5*	5*	No	6*	6*	No
Drive/Alewife Brook Parkway	Alewife Brook Parkway SB T	~39	~39	No	~23	~29	No
Parkway	Cambridgepark Drive EB	3	3	No	8*	9*	No
Alewife Brook Parkway/Rindge Avenue	Alewife Brook Parkway NB	63*	63*	No	91*	91*	No
	Alewife Brook Parkway SB	7*	4*	No	7*	7*	No
	Rindge Avenue WB L	7	7	No	7*	7*	No
	Rindge Avenue WB R	~18	~19	No	27*	27*	No
	Alewife Brook Parkway NB L	~25	~26	No	~24	~24	No
	Alewife Brook Parkway NB T	4	4	No	3	3	No
	Alewife Brook Parkway SB T	7	7	No	4	4	No
	Alewife Brook Parkway SB R	17	17	No	15	15	No
Alewife Brook Parkway	Route 2 EB L	~11	~11	No	~11	~11	No
at Route 2/16	Route 2 EB R	9	9	No	6	6	No
	Alewife Station Exit Ramp WB T	3	2	No	7	7	No
	Alewife Station Exit Ramp WB R	1	1	No	3	3	No

Note: Synchro provides queue data in feet, the table presents queue data in number of vehicles. As directed by the TIS guidelines 1 vehicle = 25 ft

Criteria E - Pedestrian Delay

		Mor	ning Peak	Hour	Evening Peak Hour		
Intersection	Crosswalk	Existing	Build	Exceeds Criteria?	Existing	Build	Exceeds Criteria?
Cambridgepark Drive/Steel	East	D	D	No	E	E	Yes
	West	D	D	No	E	E	Yes
Place	North	D	D	No	E	Е	Yes

[~] Volume exceeds capacity; queue is theoretically infinite

^{*} SimTraffic results presented instead of Synchro results

50 Cambridgepark Drive Development

Planning Board Permit Number: TBD	Planning	Board	Permit	Number:	TBD	
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		Mor	ning Peak	Hour	Ever	ning Peak I	Hour
Intersection	Crosswalk	Existing	Build	Exceeds Criteria?	Existing	Build	Exceeds Criteria?
	South	D	D	No	E	E	Yes
Cambridgepark Drive/Alewife Brook Parkway	No pedestrian facilities provided						
Alewife Brook	East	E	E	Yes	E	E	Yes
Parkway/Rindge Avenue	South	E	E	Yes	E	E	Yes
Alewife Brook Parkway at Route 2/16	East	E	E	Yes	E	E	Yes
Cambridgepark Drive/100 Cambridgepark Drive Driveway	South	Α	В	Yes	В	В	No
Cambridgepark Drive/Site West (outbound) Driveway	South	А	*	*	А	*	*
Cambridgepark Drive/Site	West	F	*	*	E	*	*
East (inbound) Driveway	South	Α	*	*	Α	*	*
Steel Place/Alewife Station	East	В	В	No	E	Е	Yes
Access Road (Route 2	West	Α	Α	No	Α	Α	No
Connector)	North	F	F	Yes	E	E	Yes

^{*} Driveway eliminated by Project

Criteria E – Pedestrian and Bicycle Facilities

Adjacent	Link (between)	Sidewalk or	Exceeds	Bicycle Facilities or	Exceeds
Street		Walkway Present	Criteria?	Right of Ways Present	Criteria?
Cambridgepark Drive	Site Driveway	Yes	No	Yes	No



Transportation Impact Study

This Transportation Impact Study for the proposed 50 Cambridgepark Drive Development (the Project) describes existing and future transportation conditions in the study area in accordance with the City of Cambridge Sixth Revision (November 28, 2011) of the Transportation Impact Study Guidelines. The study area for the TIS includes 4 signalized intersections and 5 unsignalized intersections as shown in Figure E above.

This section includes inventories of physical and operational conditions in the study area including roadways, intersections, crosswalks, sidewalks, on-street and off-street parking, transit facilities, and land uses in the study area. Transportation data that were collected and compiled are presented, including automatic traffic recorder counts, intersection turning movement counts, pedestrian and bicycle counts, vehicle crash data, and transit service data.

1 Inventory of Existing Conditions

1.a Roadways

The Project Site is located on Cambridgepark Drive, in an area referred to as the "Triangle" in North Cambridge. Cambridgepark Drive intersects Steel Place and Alewife Brook Parkway at a location east of the Project Site. Figure B, presented above, shows the roadway layout near the Project Site on Cambridgepark Drive.

1.b Intersections

The project study area included the following twelve study intersections which were presented above in Figure E and illustrated in Figures 1.b.1 through 1.b.7.

- 1. Cambridgepark Drive/100 Cambridgepark Drive Driveway
- 2. Cambridgepark Drive/Site West (outbound) Driveway
- 3. Cambridgepark Drive/Site East (inbound) Driveway
- 4. Cambridgepark Drive/Steel Place (signalized)
- 5. Cambridgepark Drive/Alewife Brook Parkway (signalized)
- 6. Alewife Brook Parkway/Rindge Avenue (signalized)
- 7. Steel Place/Alewife Station Access Road (Route 2 Connector)
- 8. Fresh Pond Rotary
- 9. Alewife Brook Parkway at Route 2/16 (signalized)

The Alewife Brook Parkway at Route 2/16 intersection is complex, and is controlled by four (4) separate, but coordinated, traffic signals, all of which are evaluated.



1.c Parking

On-Site Parking

According to the City's 1990 parking inventory, 50/54 Cambridgepark Drive was reported as having 68 employee parking spaces, although 79 spaces are shown on the ALTA survey plan. The existing office building and all supporting parking spaces will be demolished as part of this project.

Off-Site Parking

On-street parking is not available on study area streets, except for about 30 two-hour/loading spaces along the north side of Cambridgepark Drive. The majority of off-site parking in the area is accommodated in private lots or the MBTA garage. The MBTA Alewife Station parking garage, which provides approximately 2,733 parking spaces, is regularly full on most weekdays before 10 AM.

1.d Transit Services

Public Transit Services

Figure 1.d.1 illustrates existing Massachusetts Bay Transportation Authority (MBTA) services in the study area. The site is located within an eighth of a mile of Alewife Station, the terminal for Red Line and several MBTA Bus routes.

Buses terminating at Alewife Station include MBTA routes 62, 67, 76, 79, 84, 350 and 351. The passenger pickup and drop-off areas inside the MBTA parking structure provide shelter and scheduling information for all the buses. These routes provide access to and from the west along the Route 2 corridor. Only routes 62, 76 and 350 operate during the weekends and most routes run on 20 to 30-minute headways during the weekday peak hours. Routes 62, 76 and 351 provide service through Lexington towards Hanscom and Bedford. Routes 67, 79 and 84 provide service into Arlington while Route 350 provides service to Burlington.

The Red Line subway line runs on 4.5-minute headways during peak hours, with southbound trains destined for both Braintree and Ashmont. The Red Line connects with the Green Line at Park Street and the Orange Line at Downtown Crossing. Connections to all southern branch commuter rail lines and the Silver Line are made at South Station. In addition, a connection with the Fitchburg commuter rail line with a terminus at North Station is available at Porter Square station. Commuter parking spaces are available at Alewife at a rate of \$7.00 per day. Bicycle parking is also available with approximately 174 spaces in the garage.

Zipcar vehicles are available inside the garage at Alewife Garage, while one others are available on Cambridgepark Drive. Additional Zipcar spaces are expected to become available as and when certain already permitted residential projects on Cambridgepark Drive are constructed.



Private Transit Services

There are several Transportation Management Associations (TMAs) that operate private shuttle services from Alewife Station. These TMAs are non-profit organizations that provide alternative transportation to various commercial areas for member organization employees/residents. The Alewife TMA provides shuttle service via a single route to/from the nearby quadrangle neighborhood. The 128 Business Council provides nine shuttle routes, mainly serving destinations in Waltham and Lexington. The Middlesex 3 TMA provides two shuttle routes traveling to/from Bedford and Billerica. The routes are shown in Figure 1.d.2.

Additionally, Hubway and Zipcar are available in the surrounding area as shown in Figure 1.d.3.

1.e Land Use

Figure 1.e.1 illustrates land uses in the Cambridgepark Drive area surrounding the site, which also shows the existing uses on the Project Site. The area is largely characterized by commercial, R&D and office land use, and the presence of the Alewife MBTA terminal. In addition, there are residential developments (existing, under construction or approved) at 30, 130, 165, and 160 Cambridgepark Drive, and restaurant land uses within the MBTA station structure.

2 Data Collection

2.a ATR Counts

48-hour Automatic Traffic Recorder (ATR) counts were conducted on Wednesday, November 15th and Thursday, November 16th, 2017, to capture existing daily vehicle volumes within the Project study area. ATR counts were collected at the following locations (presented in Figure E), as requested in the TP&T Scoping Letter:

- Cambridgepark Drive, west of Steel Place
- Cambridgepark Drive, between Steel Place and Alewife Brook Parkway
- Steel Place, north of Cambridgepark Drive
- Alewife Brook Parkway, north of Cambridgepark Drive

Traffic volume summaries for these ATR locations are presented in Tables 2.a.1 and 2.a.2 as well as graphically in Figures 2.a.1 - 2.a.8. These data, representing the averages of data collected over two weekdays, illustrate the daily variations of traffic demands and the directional flow of traffic over the course of an average weekday. Electronic ATR data collection files are on the CD accompanying this document.



TABLE 2.A.1 EXISTING TRAFFIC VOLUME SUMMARY (NOVEMBER 2017)

		Morning Peak Hour			Evening Peak Hour		
Location	Daily ^a	Volume ^b	Kc	Peak Dir	Volume	K	Peak Dir
Cambridgepark Drive west of Steel Place	5,278	554	10%	72% WB	420	8%	72% EB
Cambridgepark Drive between Steel Place and Alewife Brook Parkway	10,383	790	8%	64% WB	990	10%	81% EB
Steel Place north of Cambridgepark Drive	6,938	642	9%	83% NB	591	9%	86% NB
Alewife Brook Parkway north of Cambridgepark Drive	39,826	2,668	7%	50% NB	2,423	6%	51% NB

a vehicles per day

b vehicles per peak hour

c percentage of daily traffic that occurs during the peak hour



TABLE 2.A.2 EXISTING AVERAGE DAILY TRAFFIC SUMMARY (NOVEMBER 2017)

		bridgepar		betwee	oridgepar en Steel Pi fe Brook P	ace and	north of	Steel Place Cambridge	e park Drive		ife Brook Pa Cambridgep	•
Start Time	EB	WB	Total	EB	WB	Total	NB	SB	Total	NB	SB	Total
12:00 AM	16	19	35	24	31	65	21	10	31	244	106	350
1:00 AM	6	11	17	17	17	34	11	6	17	109	56	165
2:00 AM	3	5	8	9	8	17	7	5	12	66	41	107
3:00 AM	3	3	6	5	5	10	2	5	7	54	62	116
4:00 AM	7	12	19	11	14	25	7	4	11	82	211	293
5:00 AM	25	47	72	59	68	127	86	29	115	304	944	1,248
6:00 AM	68	137	205	399	138	537	496	78	574	742	1,149	1,891
7:00 AM	139	239	378	408	304	712	400	176	576	1,336	1,312	2,648
8:00 AM	163	347	510	354	429	783	321	223	544	1,236	1,244	2,480
9:00 AM	131	364	495	509	282	791	533	109	642	1,155	1,243	2,398
10:00 AM	109	195	304	330	171	501	328	63	391	1,083	1,253	2,336
11:00 AM	144	131	275	289	161	450	193	71	264	1,136	1,260	2,396
12:00 PM	139	123	262	273	167	440	142	45	187	1,130	1,145	2,275
1:00 PM	131	126	257	275	154	429	151	61	212	1,184	1,062	2,246
2:00 PM	148	103	251	330	147	477	196	98	294	1,245	1,178	2,423
3:00 PM	192	95	287	497	139	636	366	91	457	1,258	1,104	2,362
4:00 PM	297	97	394	746	146	892	473	98	571	1,271	910	2,181
5:00 PM	280	133	413	801	189	990	511	80	591	1,297	922	2,216
6:00 PM	234	118	352	607	184	791	454	90	544	1,238	965	2,203
7:00 PM	159	116	275	425	178	603	268	88	356	1,217	935	2,152
8:00 PM	97	82	179	237	148	385	125	69	194	991	667	1,658
9:00 PM	56	81	137	183	143	326	107	58	165	1,005	585	1,590
10:00 PM	40	47	87	121	91	212	61	49	110	799	410	1,209
11:00 PM	32	40	72	71	65	136	43	30	73	647	243	890
Total	2,619	2,671	5,290	6,990	3,379	10,369	5,302	1,636	6,938	20,829	19,007	39,836

2.b Pedestrian and Bicycle Counts

Twelve-hour pedestrian and bicycle counts were performed on Wednesday, November 15, 2018, between 7:00AM and 7:00PM along Cambridgepark Drive, near the Project site, Pedestrian and Bicycle count data is summarized in Table 2.b.1.



TABLE 2.B.1 EXISTING 12-HOUR PEDESTRIAN AND BICYCLE VOLUMES (NOVEMBER 2018)

		Pedestri	an Volumes	Bicycle Volumes					
	North Sidewalk ¹		South	South Sidewalk		North Bike Lane ¹		South Sidewalk	
Start Time	EB	WB	EB	WB	EB	WB	EB	WB	
7:00 AM	3	2	93	69	0	0	1	0	
8:00 AM	0	0	124	140	0	2	5	0	
9:00 AM	0	0	90	105	0	1	4	0	
10:00 AM	0	0	66	90	0	1	1	0	
11:00 AM	3	1	104	96	0	1	1	1	
12:00 PM	7	4	118	81	0	1	0	0	
1:00 PM	5	3	57	69	0	0	1	1	
2:00 PM	3	5	42	42	0	0	2	0	
3:00 PM	13	12	34	35	0	0	2	0	
4:00 PM	66	49	58	46	0	2	2	1	
5:00 PM	95	85	57	67	0	1	3	0	
6:00 PM	93	122	50	57	0	1	1	0	
Total	288	283	893	897	0	10	23	3	

¹ Construction occurring on the north side of Cambridgepark Drive prohibits all users of the north sidewalk

2.c Intersection Turning Movement Counts and Queues

Turning movement counts, including vehicles, pedestrians, and bicycles, were conducted at the following study area intersections on Wednesday, November 15, 2018:

- 1. Cambridgepark Drive/100 Cambridgepark Drive Driveway
- 2. Cambridgepark Drive/Site West (outbound) Driveway
- 3. Cambridgepark Drive/Site East (inbound) Driveway
- 4. Cambridgepark Drive/Steel Place
- 5. Cambridgepark Drive/Alewife Brook Parkway
- 6. Alewife Brook Parkway/Rindge Avenue
- 7. Steel Place/Alewife Station Access Road (Route 2 Connector)

The results of these counts indicated that the peak hours for vehicular traffic in the study area are:

Morning Peak Hour, 8:00AM – 9:00AM
 Evening Peak Hour, 5:00PM – 6:00PM

As directed in the TIS scoping letter, 2016 TMC counts were used for the following study area intersections:

8. Fresh Pond Rotary (data collected October 5, 2016 from the 55 Wheeler Street TIS)

⁻Pedestrians are instructed to travel in the temporary walkway provided (located on the existing bike lane)

⁻Bicycle travel was impacted by the temporary pedestrian walkway – bikes travelling on the north side are instructed to share the travel lane with vehicle



9. Alewife Brook Parkway at Route 2/16 (data collected September 14, 2016 from the Residences at Alewife Station TIS)

Comparison of the ATR counts collect in November 2017 at Alewife Brook Parkway, north of Cambridgepark Drive with daily traffic from other area TIS ATRs collected in 2016 showed no increase. Therefore existing 2016 TMC counts were used with no growth rate adjustments.

The existing morning and evening peak hour vehicle, pedestrian, and bicycle turning movement volumes are presented in Figures 2.c.1 through 2.c.4. The raw count data is included on the accompanying CD.

VHB staff also conducted queue observations during the morning and evening peak hours at the signalized intersections on Tuesday, February 27, 2018 and Thursday, March 1, 2018. Table 2.c.1 presents the existing queue observations for the signalized study area intersections. A detailed queue analysis is provided in Section 7 of this report.

As traffic counts and queue observations conducted on different days can vary, some calibration of data and the traffic model are needed to relect actual conditions. The turning movement counts conducted in November 2017 appeared to be low as compared to previous studies (35 and 180R Cambridgepark Drive). The November 2017 counts also modeled lower traffic queues than what was observed in the field on February 27th and March 1st. In an effort to accurately model existing traffic conditions on the study area roadways, and working closely with TP&T, adjustments were made to accurately reflect existing traffic patterns in the area.



TABLE 2.C.1 SIGNALIZED INTERSECTION QUEUE OBSERVATIONS (# OF CARS)

Intersection	Lane Group	# of observed cars Morning Peak Hour	# of observed cars Evening Peak Hour (Tuesday)*	# of observed cars Evening Peak Hour (Thursday)*
	Steel Place NB L/T/R	1	0	1
	Steel Place SB L	3	7	23
Cambridgepark Drive/Steel	Steel Place SB L/T/R	3	8	21
Place*	Cambridgepark Drive EB L/T/R	5	19	32
	Cambridgepark Drive WB L/T	4	2	3
	Cambridgepark Drive WB R	1	1	1
Cambridgepark Drive/Alewife Brook Parkway	Alewife Brook Parkway NB L	2	3	-
	Alewife Brook Parkway NB T	5	7	-
	Alewife Brook Parkway SB T	28	29	-
	Cambridgepark Drive EB L	2	7	-
	Alewife Brook Parkway NB T/R	46	85+	-
Alewife Brook	Alewife Brook Parkway SB	4	7	-
Parkway/Rindge Avenue	Rindge Avenue WB L	7	4	-
	Rindge Avenue WB R	23+	23+	-
	Alewife Brook Parkway NB L	16	20	-
	Alewife Brook Parkway NB T	2	3	-
	Alewife Brook Parkway SB T	10	12	-
Alewife Brook	Alewife Brook Parkway SB R	17	20	-
Parkway at Route 2/16	Route 2 EB L	31+	31+	-
_, _ =	Route 2 EB R	37+	37+	-
	Alewife Station Exit Ramp WB T	4	15	-
	Alewife Station Exit Ramp WB R	1	2	-

Based on observations conducted by VHB on February 27 and March 1, 2018

^{*} Per the TIS Scoping Letter, queue observations were conducted on two different days (Tuesday and Thursday) during the PM peak hour for the Cambridgepark Dr/Steel Pl intersection.



2.d Crash Analysis

Study area crash data was obtained from MassDOT's records for the most recent three-year period available (January 2013 through December 2015). Analysis of the crash data is summarized in Table 2.d.1 and includes the calculated crash rates (number of reported crashes per million entering vehicles) based on the evening peak traffic volumes. A detailed summary by crash type is included in the Appendix.

TABLE 2.D.1 MASSDOT CRASH ANALYSIS (JANUARY 2013 – DECEMBER 2015)

	Total Crashes	Crashes Involving	Crashes Involving	Calculated Crash
	(3-year period)	Pedestrians	Bicycles	Rate ¹
Cambridgepark Drive/100 Cambridgepark Drive				
Cambridgepark Drive/Site West Driveway	4*	0	0	0.28
Cambridgepark Drive/Site East Driveway				
Cambridgepark Drive/Steel Place	3	0	0	0.18
Cambridgepark Drive at Alewife Brook Parkway	18	0	0	0.37
Alewife Brook Parkway/Rindge Avenue	31	3	0	0.60
Steel Place/Alewife Station Access Road	1	1	0	0.06
Fresh Pond Rotary	61	0	2	1.40
Alewife Brook Parkway at Route 2/16	61*	0	1	0.24

Source: MassDOT data

MassDOT has 6 districts within Massachusetts, and Cambridge falls under the jurisdiction of District 6. The average crash rate per million entering vehicles for District 6 is 0.70 for signalized intersections and 0.53 for unsignalized intersections. Eight of the nine study area intersections fall under the District 6 average for signalized/unsignalized intersections. Only the Fresh Pond Rotary exceeds the MassDOT average crash rate based on vehicle crashes.

The Fresh Pond Rotary reported 61 crashes during the three-year period. The majority of the crashes were angle collisions or sideswipes in the same direction involving only property damage. Five occurred in wet conditions and 56 occurred in dry conditions. Two crashes involved bicyclists.

¹ Vehicle crash rate per million entering vehicles

^{*} Number of crashes in the total intersection cluster – crash rate based on an average # of crashes in the cluster



2.e Public Transit

Transit stops and stations closest to the site are shown in Figure 1.d.1 presented above. Daily weekday ridership as well as operating hours and peak-hour headway data are provided in Table 2.e.1 for bus routes accessible from the site and for the Red Line. A more detailed transit analysis is provided in Section 10 of this report.

TABLE 2.E.1 MBTA SERVICES

Route	Origin/Destination	Hours of Operation	Weekday Ridership ¹	Peak Hour Headways
Route 62	Bedford V.A. Hospital – Alewife Station	5:47AM – 9:04PM	1,345	~ 30 minutes
Route 67	Turkey Hill – Alewife Station	5:53AM – 8:32PM	668	~ 24-29 minutes
Route 76	Hanscom/Lincoln Lab – Alewife Station	6:00AM – 10:39PM	874	~ 25-36 minutes
Route 79	Arlington Heights – Alewife Station	6:35AM – 10:03PM	881	~ 20-30 minutes
Route 84	Arlmont Village – Alewife Station	6:42AM – 6:59PM	337	~ 17-38 minutes
Route 350	North Burlington – Alewife Station	6:04AM – 11:00PM	1,634	~ 20-30 minutes
Route 351	EMD Serono/Bedford Woods – Alewife Station	6:15AM – 9:30AM & 3:20PM – 7:01PM	173	~ 50-60 minutes
Red Line ²	Alewife/Ashmont- Braintree Combined	5:05AM - 1:05AM	276,167	4.5 minutes

Sources: MBTA Schedule Winter 2018

2.f Parking

The existing office building on the Project Site is currently active (open for business) and is supported by approximately 68 parking spaces, per the City's 1990 parking inventory. Because the building and parking lot will be demolished as part of the Project, a parking utilization study was not conducted for the existing building.

 $^{^{\}rm 1}$ MBTA provided ridership data (Fall 2017 for buses; Fall 2016 Red line)

² Ashmont/Braintree Ridership Data is combined



3 Project Traffic

3.a Mode Share and Vehicle Occupancy Rate

In coordination with the City of Cambridge, Traffic, Parking and Transportation Department (TP&T), residential mode shares for the Project were developed from data based on 160 Cambridgepark Drive 2017 TDM monitoring report. Retail/restaurant mode shares are based on discussions with TP&T, along with the 2015 Alewife intercept study and the 160 Cambridgepark Drive 2017 TDM monitoring report. Table 3.a.1 presents the TP&T approved mode share rates for this analysis.

TABLE 3.A.1 MODE SHARE

Mode	Project Residential Use	Project Retail/ Restaurant Use
SOV	28%	18%
HOV	2%	2%
Transit	51%	20%
Bike	5%	5%
Walk	8%	52%
Other	6%	3%
Total	100%	100%

The Federal Highway Administration 2009 National Household Travel Survey Summary of Travel Trends provided the national vehicle occupancy rates (VOR) of 1.13 for work trips which are used to convert Institute of Transportation Engineers (ITE) unadjusted vehicle trips to person trips. Two local VORs were used for the Project. The SOV VOR is 1.0 while the HOV VOR was calculated to be 2.08 based on data from the 2012-2016 American Community Survey (ACS) 5 Year Estimates for the census tract 3549, Middlesex County, MA.

3.b Trip Generation and Trip Credit for Existing Use on Site

In an effort to provide the most accurate trip generation estimates for the proposed project, each proposed land use (residential and retail/restaurant) was examined individually.

Per the City's scoping letter, instead of using the ITE *Trip Generation Manual* (9th Edition) rates for Apartments (LUC 220), the residential trip generation analysis is based on observed vehicle trip rates from the comparable and adjacent 130 Cambridgepark Drive residential building.

In coordination with the TP&T, a methodology for vehicle trip generation was developed using a combination of the trip rates from ITE for Apartments (LUC 220) adjusted for local mode split of 28% SOV and 2% HOV and observed 130 Cambridgepark Drive vehicle trips rates (from both the 130 and 140 Cambridgepark Drive garages).

Table 3.b.1 presents the TP&T approved trip rates for this analysis.



TABLE 3.B.1 VEHICLE TRIP RATES

	ITE Adjusted (LUC 220)	130 Cambridgepark Dr (February 2018)	Rates Used for the Project
Morning Peak Hour			
In	0.03	0.03	0.03
Out	0.13	0.13	0.13
+			
Evening Peak Hour			
In	0.13	0.12	0.12
Out	0.07	0.05	0.05

ITE Trip Generation Manual 9th Edition LUC 220 - Apartment

Rates for 130 Cambridgepark Drive based on Hanover Company data from February 2018 (based on occupied units)

For other travel modes (transit, walk, bike and other), the mode shares (28% SOV and 2% HOV) along with the VORs were applied to the vehicle trip rate to determine the total project person generation estimate. These persons trip were then distributed, per the mode shares, to each commuting option. Table 3.b.2, below, shows the residential project generated trips (before existing use credit) using the proposed trip rates shown in Table 3.B.1.

TABLE 3.B.2 PROJECT RESIDENTIAL USE GENERATED TRIPS (BEFORE EXISTING USE CREDIT)

		Vehicle Tri	os		Transit Trip	os		Bicycle Trip	S		Walk Trips	5
	Daily	Morning Peak	Evening Peak	Daily	Morning Peak	Evening Peak	Daily	Morning Peak	Evening Peak	Daily	Morning Peak	Evening Peak
Entering	206	9	36	363	16	63	36	2	6	57	2	10
<u>Exiting</u>	<u>206</u>	<u>39</u>	<u>14</u>	<u>363</u>	<u>68</u>	<u>26</u>	<u>36</u>	<u>7</u>	<u>3</u>	<u>57</u>	<u>11</u>	<u>4</u>
Total	312	48	50	726	84	89	72	9	9	114	13	14

For the retail/restaurant use, many ITE *Trip Generation Manual* land use codes (LUC) were examined to determine which would be the best fit for the area. Per the City's scoping letter and after consideration of various Institute of Transportation Engineers (ITE) Trip Generation rates, it was decided that High-Turnover Restaurant (LUC 932) was the most appropriate as it best matches the size of the retail/restaurant space proposed for this project compared to other commercial trip generation rates.

Table 3.b.3 summarizes the retail/restaurant project generated trips (before existing use credit) by mode.



TABLE 3.B.3 PROJECT RETAIL/RESTAURANT USE GENERATED TRIPS (BEFORE Ex. USE CREDIT)

		Vehicle Tri	ps		Transit Trip	os		Bicycle Trip	S		Walk Trips	5
	Daily	Morning Peak	Evening Peak	Daily	Morning Peak	Evening Peak	Daily	Morning Peak	Evening Peak	Daily	Morning Peak	Evening Peak
Entering	95	8	8	100	9	9	25	2	2	260	24	24
<u>Exiting</u>	<u>95</u>	<u>7</u>	<u>6</u>	<u>100</u>	<u>8</u>	<u>6</u>	<u>25</u>	<u>2</u>	<u>2</u>	<u>260</u>	<u>20</u>	<u>16</u>
Total	190	15	14	200	17	15	50	4	4	520	44	40

The total project trip generation estimate is a combination of the two land uses trip generation estimates presented in Tables 3.b.2 and 3.b.3. The resulting project trip generation by mode for the proposed project is summarized in Table 3.b.4.

TABLE 3.B.4 TOTAL PROJECT GENERATED TRIPS (BEFORE EXISTING USE CREDIT)

		Vehicle Tri	ps		Transit Trip	os		Bicycle Trip	S		Walk Trips	5
	Daily	Morning Peak	Evening Peak	Daily	Morning Peak	Evening Peak	Daily	Morning Peak	Evening Peak	Daily	Morning Peak	Evening Peak
Entering	301	17	44	463	25	72	61	4	8	317	26	34
<u>Exiting</u>	<u>301</u>	<u>46</u>	<u>20</u>	<u>463</u>	<u>76</u>	<u>32</u>	<u>61</u>	<u>9</u>	<u>5</u>	<u>317</u>	<u>31</u>	<u>20</u>
Total	602	63	64	926	101	104	122	13	13	634	57	54

As approved by TP&T, the analysis includes a vehicle trip generation credit for existing on-site uses. The existing approximately 39,000 square foot office buildings will be demolished as part of the project. Accordingly, vehicle trips associated with the existing buildings will be removed from the roadway network.

Counts conducted on November 15, 2017 at the existing site driveway were compared with estimates using ITE trip rates to determine the trip credit for eliminating the existing use on the site, as summarized in Table 3.b.5. ITE LUC 760 – Research and Development Center was used for the estimate. The ITE vehicle trip generation estimate was adjusted based on metrics from the 35 Cambridgepark Drive Transportation Study (61% auto mode share and VOR of 1.13).



TABLE 3.B.5 VEHICLE TRIP CREDIT

	ITE Adjusted (LUC 760)	Ex Driveway Counts (November 2017)
Morning Peak Hour		
In	29	18
Out	6	0
Evening Peak Hour		
In	6	3
Out	31	14

Source:

ITE Trip Generation Manual, 9th Edition LUC 760 – Research and Development Center Existing driveway counts conducted on Nov. 15, 2017

As seen in Table 3.b.5, the driveway counts produce a lower vehicle trip estimate as compared to ITE. Therefore, it was determined that the vehicle trip credit should be based on actual driveway counts. Trip credits (i.e. trips to be removed from roadway network due to removal of office building form existing site) and net-new project trips are presented in Table 3.b.6.

TABLE 3.B.6 NET-NEW PROJECT GENERATED VEHICLE TRIPS

	Project Generated Trips (Table 3.B.4)	Existing Site Generated Trips	Net New Trips
Morning Peak Hour			
In	17	-18	-1
Out	46	0	46
Evening Peak Hour			
In	44	-3	41
Out	20	-14	6

3.c Trip Distribution and Assignment

Two vehicle distributions were used for this analysis. For project trips, work being done as part of the Envision Citywide Cambridge planning study was used as a basis to determine distribution of project vehicle trips onto the roadway network, as directed by the TP&T. Table 3.c.1 and Figure 3.c.1 summarize the project vehicle trip distribution.



TABLE 3.C.1 SUMMARY OF PROJECT VEHICLE TRIP DISTRIBUTION

		Distr	ibution
Trip Assignment	Direction	Inbound	Outbound
Route 2	To/From Northwest	10%	15%
Route 16	To/from Northeast	20%	28%
Route 16 (Fresh Pond Parkway)/ Concord Avenue	To/from South	35%	35%
Concord Avenue	To/From West	22%	22%
Rindge Avenue	To/From East	13%	0%

Source: Envision Citywide Cambridge Planning Study

As the existing site use (office/research use) is different from the proposed use, a separate distribution was used to remove the existing site trips from the roadway network before adding in the proposed project trips. This trip credit distribution is based on 35 Cambridgepark Drive Transportation Study from October 2016. Table 3.c.2 and Figure 3.c.2 summarize the existing site credit vehicle trip distribution.

TABLE 3.C.2 SUMMARY OF EXISTING SITE CREDIT VEHICLE TRIP DISTRIBUTION

		Distr	ibution
Trip Assignment	Direction	Inbound	Outbound
Route 2	To/From Northwest	52%	47%
Route 16	To/from Northeast	18%	24%
Route 16 (Fresh Pond Parkway)	To/from South	21%	25%
Concord Avenue	To/From West	4%	4%
Rindge Avenue	To/From East	5%	0%

Source: 35 Cambridgepark Drive Transportation Study, October 2016

Project vehicle trips and existing trip credits were assigned to the roadway network using the appropriate distribution and are presented in the Project Generated network figures. Because the site has an active existing use, both "Total" Project Generated Trips (only project vehicle trips) as well as "Net-New" Project Generated Trips (project vehicle trips minus existing trip credit), are presented graphically in Figures 3.c.3 through 3.c.6.

3.d Service and Loading

The proposed project is expected to generate a limited number of delivery trips over the course of a normal day. Typical deliveries will include mail and trash collection for the building as a whole. These types of deliveries will be directed to use the loading dock along the shared driveway. As this is a residential building, move-in/move-out activity will occur occasionally. Depending on the size of the vehicle, move-in/move-out activity can occur in the loading dock or within a dedicated loading area within the garage. Building management will actively schedule move-in/move-out activity with tenants to ensure multiple tenants are not moving at the same time. Proposed service and loading facilities are presented in Figure 3.d.1. and truck



turns for the loading dock are shown in Figure 3.d.2. The design of the sidewalk and streetscape will be carefully developed in coordination with TP&T to ensure adequate sightlines at the sevice and garage curb-cuts.

Typically, residential trash will be picked up two times per week. Move-ins are expected to be more frequent in the first 12 months of building occupancy, and taper off after that time period.

4 Background Traffic

In accordance with the City's Scoping Letter and TIS Guidelines, a general background traffic growth of 0.5% per year for five years to the 2023 Future Condition was included in the Future condition analysis.

In addition, trips associated with specific planned projects in the area of the Project site have been incorporated into the 2023 Future Condition analysis. These specific projects include:

- 605 Concord Avenue
- 87-95 Fawcett Street
- 75 New Street
- 130 Cambridgepark Drive
- 88 Cambridgepark Drive
- 35 Cambridgepark Drive renovation project
- 55 Wheeler Street
- The Residences at Alewife Station (195 & 211 Concord Turnpike)

5 Traffic Analysis

Morning and Evening peak hour traffic networks were developed in accordance with the TIS Guidelines, for the 2018 Existing, 2018 Build and 2023 Future Condition scenarios.

5.a 2018 Existing Condition

The 2018 Existing Condition analysis is based on existing vehicle, bicycle, and pedestrian counts at the study area intersections (see Section 2). The Existing Condition networks are shown in Figures 2.c.1 through 2.c.4 presented above.

5.b 2018 Build Condition

The 2018 Build Condition assumes full occupancy of 299 residential units. Since the counts for the Existing Condition were completed while the existing office building was still occupied as offices, these driveway trips were subtracted from the network before the project-generated trips were added to the network. Therefore, the resulting 2018 Build network consists of the



2018 Existing volumes plus the net-new project generated trips. These networks are shown in Figures 4.c.1 and 4.c.2.

5.c 2023 Future Condition

Background traffic growth was assumed to occur at 0.5 percent per year for five years to the 2023 Future Condition. Additionally, volumes generated from background projects that are planned to come on-line during this five-year period were added to the network. The 2023 Future Condition networks are shown in Figures 5.c.1 and 5.c.2. In addition, Figure 5.c.3. shows evening cumulative impacts on study are roadways inclusive of both the proposed project as well as background projects planned to come on-line during the five-year period.

6 Vehicle Capacity Analysis

6.a Capacity Analysis

Synchro 9 software was used to determine the vehicle level of service (VLOS) for the ten signalized and unsignalized study area intersections. Synchro software is based on the 2000 Highway Capacity Manual. Because of Synchro's limitations when analyzing rotaries SIDRA 7 software was used for the Fresh Pond Rotary to determine the vehicle level of service. SIDRA software is based on the 2010 Highway Capacity Manual.

Results for the 2018 Existing, 2018 Build, and 2023 Future Conditions are presented in Table 6.a.1 and Table 6.a.2 for signalized intersections, Table 6.a.3 and Table 6.a.4 for unsignalized intersections, and Table 6.a.5 and Table 6.a.6 for the Fresh Pond Rotary. The tables also show the difference in delay between the Existing and Build conditions (delay due to project impact) and between the Existing and Future delay (total delay from project and other background growth). Figures 6.a.1 and 6.a.2 illustrate the overall VLOS and Figures 6.b.1 and 6.b.2 illustrate the net change in delay for each intersection for the morning and evening peak hour respectively. A summary of the analysis results follows.

The existing conditions of the signalized intersections during the morning peak hour operate at an LOS C or better with the exception of Alewife Brook Parkway at Rindge Ave which operates at an LOS F and Alewife Brook Parkway at Route 2/16 and Cambridgepark Drive at Alewife Brook Parkway which operate at an LOS E. The unsignalized intersections primarily operate at a LOS C or better with the exception of Steel Place at Alewife Station Access Road which operated at an LOS F.

The existing conditions of the signalized intersections during the evening peak hour operate at an LOS C or better with the exception of Alewife Brook Parkway at Rindge Ave, Alewife Brook Parkway at Route 2/16 and Cambridgepark Drive at Alewife Brook Parkway which operate at an LOS F. The unsignalized intersections primarily operate at a LOS C or better with the exception of Steel Place at Alewife Station Access Road which operated at an LOS F.



During both the morning and evening peak hour, the project impacts are no greater than 10 seconds of delay at each of the study area intersections, and LOS at each intersection does not decline.



Table 6.a.1 Signalized Intersection Level of Service Results – Moring Peak Hour

		Ü	Existing (20)	2018)		Builk	Build (2018)			Future	Future (2023)	
Intersection	Movement	v/c	Delay	VLOS	v/c	Delay	VLOS	Difference in Delay	v/c	Delay	VLOS	Difference in Delay
	Cambridgepark Drive EB Left/Thru/Right	0.41	25.9	C	0.58	30.9	J	5.0	96.0	69.5	Е	43.6
	Cambridgepark Drive WB Left/Thru	0.78	37.7	Δ	0.79	38.7	Δ	1.0	0.85	43.0	۵	5.3
Cambridgepark	Cambridgepark Drive WB Right	0.18	23.2	U	0.18	23.2	U	0.0	0.21	23.8	U	9:0
Drive/Steel Place	Steel Place NB Left/Thru/Right	0.21	37.8	Δ	0.21	37.8	Δ	0.0	0.21	37.8	۵	0.0
	Steel Place SB Left	0.40	26.8	U	0.40	26.8	U	0.0	0.43	27.4	U	9.0
	Steel Place SB Thru/Right	0.42	30.2	U	0.41	29.7	U	-0.5	0.43	30.2	U	0.0
	Overall	0.54	31.4	U	0.54	32.5	U	1.1	0.62	42.7	۵	11.3
	Cambridgepark Drive EB Left/Right	0.35	37.8	۵	0.40	38.5	Δ	0.7	0.52	40.1	۵	2.3
Jacobridge	Alewife Brook Parkway NB Left	1.09	94.1	ட	1.11	97.0	ш	2.9	1.24	152.7	ட	58.6
Drive/Alewife	Alewife Brook Parkway NB Thru	0.94	14.3	В	0.94	12.7	В	-1.6	86.0	15.2	Ф	6.0
Brook Parkway	Alewife Brook Parkway SB Thru	1.19	129.9	ш	1.19	129.9	ш	0.0	1.24	149.1	ш	19.2
	Alewife Brook Parkway SB Right	0.23	0.4	۷	0.23	0.4	4	0.0	0.24	0.4	∢	0.0
	Overall	0.95	62.0	ш	0.97	61.4	ш	-0.6	1.06	72.6	ш	10.6
	Rindge Avenue WB Left	0.98	109.0	ш	0.98	109.0	ш	0.0	1.00	116.3	ш	7.3
- :	Rindge Avenue WB Right	1.89	473.7	ட	1.89	476.0	ц	2.3	2.05	546.4	ட	72.7
Alewite Brook Parkway/Rindge	Alewife Brook Parkway NB Thru/Right	0.85	21.3	U	0.85	21.5	U	0.2	0.89	24.5	U	3.2
Avenue	Alewife Brook Parkway SB Thru	1.04	31.1	U	1.05	38.3	Δ	7.2	1.12	70.0	ш	38.9
	Overall	1.12	86.9	ц	1.13	90.1	ш	3.2	1.21	114.4	щ	27.5



		ú	Existing (2018)	18)		Builc	Build (2018)			Future	Future (2023)	
Intersection	Movement	5//	Delay	V ()	2//	Delay	VO IX	Difference in	2//\	Delay	VOIA VOIA	Difference in
		2	(cla))	, see a see	,	2	2/2	, and a second		, and a
Alewife Brook	Alewife Station Access Road WB	000	<u></u>	<	600	U	<	5	90 0	0	<	90
Parkway at	ח	0.00	4.0	(0.03	0.0	τ	T:0	0.00	0:0	(0.0
Route 2/16 –	Alewife Brook Parkway SB Right	0.89	38.2	Δ	0.89	38.2	Δ	0:0	0.91	40.3	Ω	2.1
Signal A	Overall	0.91	18.4	8	0.91	18.4	ω	0.0	0.94	19.5	ω	11
	Concord Turnpike EB Left	1.06	104.9	ட	1.06	104.9	ш	0.0	1.10	119.3	ш	14.4
Alewife Brook	Alewife Station Access Road WB											
Parkway at	Thru	0.29	14.5	В	0.30	14.5	В	0.0	0.32	14.8	В	0.3
Route 2/16 -	Alewife Brook Parkway SB Thru	0.55	41.8	Δ	0.55	41.8	Ω	0.0	0.58	42.4	Δ	9.0
Signal B	Alewife Brook Parkway NB Left	1.02	61.7	ш	1.02	62.4	ш	0.7	1.06	73.4	ш	11.7
	Overall	0.89	64.2	ш	0.89	64.6	ш	0.4	0.93	73.6	ш	9.4
	Alewife Station Access Road WB											
Alewife Brook	Thru	0.20	8.5	∢	0.20	8.5	∢	0.0	0.22	9.8	⋖	0.1
Parkway at	Alewife Station Access Road WB											
Route 2/16 –	Right	0.08	7.5	⋖	0.08	9.7	∢	0.1	0.11	7.8	⋖	0.3
Signal C	Alewife Brook Parkway NB Thru	0.33	37.8	Δ	0.34	37.9	Δ	0.1	0.38	38.6	Δ	0.8
	Overall	0.25	22.9	U	0.25	22.9	U	0.0	0.27	23.2	U	0.3
Alewife Brook	Alewife Brook Parkway SB Thru	0.57	7.1	∢	0.57	7.1	∢	0.0	09.0	7.3	⋖	0.2
Parkway at	Concord Turnpike EB Right	0.53	11.7	В	0.53	11.7	В	0.0	0.55	12.0	В	0.3
Route 2/16 - Signal D	Overall	0.57	10.4	ω	0.57	10.4	ω	0.0	0.59	10.7	ω	0.3
)		L										

v/c = volume-to-capacity ratio; Delay = average delay expressed in seconds per vehicle; VLOS = vehicular level of service

28



TABLE 6.A.2 SIGNALIZED INTERSECTION LEVEL OF SERVICE RESULTS - EVENING PEAK HOUR

		 	Existing (2018)	.8)		Build	Build (2018)			Future	Future (2023)	
Intersection	Movement	3//	Delay	SOIA	3//	Delav	SOIN	Difference in	5//	Delay	SOIN	Difference in
	Cambridgepark Drive EB											
	Lett/Inru/Rignt	0.70	32.0	ر	0.72	32.7	ر	0.7	0.86	44.1	ے	17.1
	Cambridgepark Drive WB Left/Thru	0.43	24.3	U	0.51	25.9	U	1.6	0.61	28.8	U	4.5
Cambridgepark	Cambridgepark Drive WB Right	60.0	19.7	В	60.0	19.7	В	0.0	0.10	20.0	В	0.3
Drive/Steel Place	Steel Place NB Left/Thru/Right	0.28	40.0	Δ	0.28	40.0	۵	0.0	0.35	40.6	۵	9.0
	Steel Place SB Left	0.64	30.8	U	0.65	31.2	U	0.4	0.68	32.5	U	1.7
	Steel Place SB Thru/Right	0.71	34.7	U	0.71	34.5	U	-0.2	0.76	38.0	Δ	3.3
	Overall	99.0	31.1	U	99.0	31.4	U	0.3	0.76	35.8	۵	4.7
	Cambridgepark Drive EB Left/Right	1.05	72.9	ш	1.06	75.2	ш	2.3	1.12	97.6	ш	24.7
Cambridgepark	Alewife Brook Parkway NB Left	0.95	54.8	Δ	1.12	102.3	ш	47.5	1.39	209.4	ш	154.6
Drive/Alewife	Alewife Brook Parkway NB Thru	0.98	21.5	U	86.0	21.1	U	-0.4	1.01	23.4	U	1.9
Brook Parkway	Alewife Brook Parkway SB Thru	1.44	243.5	ш	1.44	243.5	ш	0.0	1.50	270.0	ш	26.5
	Alewife Brook Parkway SB Right	0.07	0.1	⋖	80.0	0.1	4	0.0	60.0	0.1	∢	0.0
	Overall	1.28	107.1	ц	1.28	109.2	ц	2.1	1.36	130.1	ц	23.0
	Rindge Avenue WB Left	0.59	43.6	Δ	0.59	43.6	۵	0.0	09:0	44.2	۵	9:0
:	Rindge Avenue WB Right	09.0	44.4	Ω	0.63	45.5	۵	1.1	0.81	59.3	ш	14.9
Alewife Brook	Alewife Brook Parkway NB											
Parkway/kiiiuge Avenije	Thru/Right	0.99	9.09	Δ	1.01	55.0	Δ	4.4	1.05	67.5	ш	16.9
	Alewife Brook Parkway SB Thru	1.42	204.9	ш	1.42	207.8	ш	2.9	1.50	240.5	щ	35.6
	Overall	0.94	125.4	ц	0.95	128.1	ш	2.7	1.03	150.1	ш	24.7



		Ð	Existing (2018)	(8)		Build	Build (2018)			Future	Future (2023)	
Intersection	Movement	\/c	Delay	VLOS	۸/د	Delay	VLOS	Difference in Delay	۸/د	Delay	VLOS	Difference in Delay
Alewife Brook Parkway at	Alewife Station Access Road WB Thru	0.97	6.7	∢	0.97	6.6	∢	-0.1	1.01	12.8	B	6.1
Route 2/16 –	Alewife Brook Parkway SB Right	0.95	42.4	۵	0.95	42.4	Δ	0.0	0.97	46.8	۵	4.4
Signal A	Overall	1.03	1.61	8	1.03	19.0	8	-0.1	1.06	24.5	U	5.4
	Concord Turnpike EB Left	1.31	197.4	ш	1.31	197.4	ட	0.0	1.35	213.2	ட	15.8
Alewife Brook	Alewife Station Access Road WB	101	1006	Ц	1 2 5	1006	Ц	c	, r	7000	Ц	000
Parkway at Route 2/16 –	Alewife Brook Parkway SB Thru	0.48	34.8	. U	0.49	35.0	- U	0.5	0.52	35.5	- 🗅	0.7
Signal B	Alewife Brook Parkway NB Left	1.18	117.2	ш	1.18	115.9	ш	-1.3	1.22	132.1	ш	14.9
	Overall	1.10	137.0	ш	1.10	136.1	ш	6.0-	1.15	152.7	ш	15.7
Alawifa Brook	Alewife Station Access Road WB Thru	0.54	11.5	В	0.54	11.5	Ω	0.0	0.57	12.0	Ω	0.5
Parkway at Route 2/16 –	Alewife Station Access Road WB Right	0.37	9.2	∢	0.37	9.2	∢	0.0	0.39	9.5	∢	0.3
Signal C	Alewife Brook Parkway NB Thru	0.35	32.7	U	0.35	32.8	U	0.1	0.37	33.1	U	0.4
	Overall	0.52	15.9	8	0.52	15.9	8	0.0	0.55	16.3	8	0.4
Alewife Brook	Alewife Brook Parkway SB Thru	0.50	6.7	٨	0.51	8.9	∢	0.1	0.54	7.0	٨	0.3
Parkway at	Concord Turnpike EB Right	0.44	9.3	٨	0.44	9.3	∢	0.0	0.46	9.4	A	0.1
Koute 2/16 – Signal D	Overall	0.48	8.5	4	0.49	8.5	4	0.0	0.51	8.7	4	0.2

v/c = volume-to-capacity ratio; Delay = average delay expressed in seconds per vehicle; VLOS = vehicular level of service



Table 6.a.3 Unsignalized Intersection Level of Service Results – Morning Peak Hour

		<u> </u>	Existing (2018)	18)		Builc	Build (2018)			Fut	Future (2023)	3)
Intersection	Approach	v/c	v/c Delay	VLOS	v/c	Delay	VLOS	Difference in Delay	v/c	Delay VLOS	VLOS	Difference in Delay
Cambridgepark Drive/100 Cambridgepark Drive Driveway	100 Cambridgepark Drive Driveway NB	0.12	17.8	C	0.25	0.25 16.9	C	-0.9	0.46	0.46 20.0	C	2.2
Cambridgepark Drive/Site West (outbound) Driveway	Site West (outbound) Driveway NB	0.00	0.0	∢	ı	ı	ı	1	ı	1	ı	ı
Cambridgepark Drive/Site East (inbound) Driveway	Cambridgepark Drive WB	0.02	0.5	∢	ı	Y	ı	ı	ı	ı	ı	ı
Steel Place/Alewife Station Access Road (Route 2 Connector)	Alewife Station Access Road SB	1	153.9	ш	1	151.3	ч	-2.6	ı	188.2	ъ	36.9

Table 6.a.4 Unsignalized Intersection Level of Service Results – Evening Peak Hour

		Ĥ	Existing (2018)	018)		Buile	Build (2018)			Fut	Future (2023)	a
Intersection	Approach	v/c	v/c Delay	VLOS	v/c	Delay	VLOS	Difference in Delay	v/c	Delay	VLOS	Difference in Delay
Cambridgepark Drive/100 Cambridgepark Drive Driveway (Site Driveway in Build and Future Condition)	100 Cambridgepark Drive Driveway NB	0.24	0.24 12.8	Ф	0.28	24 12.8 B 0.28 13.3 B	Ф	0.5	0.34	0.34 14.2	ω	1.4
Cambridgepark Drive/Site West (outbound) Driveway	Site West (outbound) Driveway NB	90.0	12.7	В	ı	ı	ı	ı	ı	1	1	1
Cambridgepark Drive/Site East (inbound) Driveway	Cambridgepark Drive WB	0.00	0.0	∢	ı	ı	ı	ı	ı	ı	ı	ı
Steel Place/Alewife Station Access Road (Route 2 Connector)	Alewife Station Access Road SB	1	93.5	щ	1	93.5	Щ	0.0	1	116.4	Щ	22.9



TABLE 6.a.5 ROTARY LEVEL OF SERVICE RESULTS – MORNING PEAK HOUR

		Exist	xisting (2018)			Build	Build (2018)			Future	Future (2023)	
Intersection	Approach	$Demand^1$	Delay	VLOS	VLOS Demand Delay	Delay	VLOS	Difference in Delay	Demand Delay	Delay	VLOS	Difference in Delay
Fresh Pond Rotary	Concord Ave WB	1,920	144.4	ட	1,923	145.7	ட	1.3	2,020	160.0	ഥ	15.6
	Hotel Driveway SWB 80	80	19.0	U	19.0 C 80 19.0 C	19.0	U	0.0	82	82 20.2 C	U	1.2
	Alewife Brook Pkwy SB	1,566	168.3	Ш	1,592	177.2	Ш	8.9	1,702	241.9	Щ	73.6
Concord Ave EB	Concord Ave EB	995	83.1	ட	866	82.9	ட	-0.2	1,135	118.6	ഥ	35.5
	Overall	4,561	137.0	F	4,593	140.8	F	3.8	4,956	176.4	F	49.4
		-										

Approach volume in vehicles per hour

TABLE 6.A.6 ROTARY LEVEL OF SERVICE RESULTS – EVENING PEAK HOUR

		Exist	xisting (2018)			Build	Build (2018)			Futur	Future (2023)	
Intersection	Approach	$Demand^1$	Delay	VLOS	Demand	Delay	VLOS	Difference in Delay	Demand	Delay	VLOS	Difference in Delay
Fresh Pond Rotary	Fresh Pond Rotary Concord Ave WB 1,342	1,342	47.7	ш	1,361	53.9	L	6.2	1,492	62.5	Ш	14.8
	Hotel Driveway SWB 42	42	11.7	В	42	11.7	В	0.0	44	12.8	В	1.1
	Alewife Brook Pkwy SB	1,828	96.3	Щ	1,836	98.2	Щ	1.9	1,932	158.8	ш	62.5
	Concord Ave EB	778	79.3	Щ	789	82.2	ட	2.9	871	86.4	Щ	7.1
	Overall	3,990	75.8	ч	4,028	79.2	4	3.4	4,340	109.7	ч	33.9

¹ Approach volume in vehicles per hour



7 Queue Analysis

Queue analysis was performed in combination with the LOS analysis. Tables 7.a.1 and 7.a.2 show the results for the modeled average queues (expressed in the number of vehicles) for each scenario for the morning and evening peak hour, respectively. Because of the limitations of Synchro, when two intersections are close to each other due to recognizing queue backups, Sim Traffic was used to evaluate queueing.

VHB staff conducted queue observations during the morning and evening peak hours at the signalized intersections on Tuesday, February 27th, 2018 and during the evening peak hour on Thursday, March 1st, 2018 at Cambridge Park Drive at Steel Place. Cambridgepark Drive at Steel Place queues in the evening are presented in Table 7.a.3 in order to identify variations in the study day queues.

TABLE 7.A.1 SIGNALIZED INTERSECTION QUEUE ANALYSIS - MORNING PEAK HOUR

Intersection	Lane Group	A	verage Queu	e in Vehicles	
		2018 Observed	2018 Existing Modeled	2018 Build Modeled	2023 Future Modeled
	Steel Place NB L/T/R	1	1	1	1
	Steel Place SB L	3	4	4	5
Cambridgepark	Steel Place SB L/T/R	3	1	1	1
Drive/Steel Place	Cambridgepark Drive EB L/T/R	5	4	5	9
	Cambridgepark Drive WB L/T	4	10	11	12
	Cambridgepark Drive WB R	1	0	0	0
	Alewife Brook Parkway NB L	2	4*	5*	5*
Cambridgepark Drive/Alewife Brook	Alewife Brook Parkway NB T	5	5*	5*	5*
Parkway	Alewife Brook Parkway SB T	28	~39	~39	~41
	Cambridgepark Drive EB	2	3	3	5
	Alewife Brook Parkway NB	46	63*	63*	63*
Alewife Brook	Alewife Brook Parkway SB	4	7*	4*	5*
Parkway/Rindge Avenue	Rindge Avenue WB L	7	7	7	7
	Rindge Avenue WB R	23+	~18	~19	~21
	Alewife Brook Parkway NB L	16	~25	~26	~27
	Alewife Brook Parkway NB T	2	4	4	4
	Alewife Brook Parkway SB T	10	7	7	7
Alewife Brook Parkway	Alewife Brook Parkway SB R	17	17	17	18
at Route 2/16	Route 2 EB L	31+	~11	~11	~11
	Route 2 EB R	37+	9	9	10
	Alewife Station Exit Ramp WB T	4	3	2	3
	Alewife Station Exit Ramp WB R	1	1	1	1

Note: Synchro provides queue data in feet, the table presents queue data in number of vehicles. As directed by the TIS guidelines 1 vehicle = 25 ft

Morning queue observations conducted at signalized intersections on Tuedsay, February 27, 2018.

[~]Volume exceeds capacity; queue is theoretically infinite.

^{*}SimTraffic results presented instead of Synchro results



TABLE 7.A.2 SIGNALIZED INTERSECTION QUEUE ANALYSIS - EVENING PEAK HOUR

Intersection	Lane Group	A	verage Queu	e in Vehicles	
		2018 Observed	2018 Existing Modeled	2018 Build Modeled	2023 Future Modeled
	Alewife Brook Parkway NB L	3	4*	4*	5*
Cambridgepark	Alewife Brook Parkway NB T	7	6*	6*	6*
Drive/Alewife Brook Parkway	Alewife Brook Parkway SB T	29	~23	~29	~31
Tarkway	Cambridgepark Drive EB	7	8*	9*	9*
	Alewife Brook Parkway NB T	85+	91*	91*	91*
Alewife Brook	Alewife Brook Parkway SB	7	7*	7*	8*
Parkway/Rindge Avenue	Rindge Avenue WB L	4	7*	7*	7*
	Rindge Avenue WB R	23+	27*	27*	27*
	Alewife Brook Parkway NB L	20	~24	~24	~26
	Alewife Brook Parkway NB T	3	3	3	3
	Alewife Brook Parkway SB T	12	4	4	5
Alewife Brook Parkway	Alewife Brook Parkway SB R	20	15	15	16
at Route 2/16	Route 2 EB L	31+	~11	~11	~11
	Route 2 EB R	37+	6	6	6
	Alewife Station Exit Ramp WB T	15	7	7	8
	Alewife Station Exit Ramp WB R	2	3	3	4

Note: Synchro provides queue data in feet, the table presents queue data in number of vehicles. As directed by the TIS guidelines 1 vehicle = 25 ft

Evening queue observations conducted at signalized intersections on Tuedsay, February 27, 2018.

TABLE 7.A.3 CAMBRIDGEPARK DRIVE AT STEEL PLACE QUEUE ANALYSIS - EVENING PEAK HOUR

Intersection	Lane Group		А	verage Queu	e in Vehicles	1	
		Tuesday Observed	Thursday Observed	2018 Observed (Average) ¹	2018 Existing Modeled	2018 Build Modeled	2023 Future Modeled
	Steel Place NB L/T/R	0	1	1	1	1	1
	Steel Place SB L	7	23	13	7	7	7
Cambridgepark	Steel Place SB L/T/R	8	21	13	7	7	8
Drive/Steel Place	Cambridgepark Drive EB L/T/R	19	32	24	8	8	10
	Cambridgepark Drive WB L/T	2	3	3	4	5	7
	Cambridgepark Drive WB R	1	1	1	0	0	0

Note: Synchro provides queue data in feet, the table presents queue data in number of vehicles. As directed by the TIS guidelines 1 vehicle = 25 ft

Evening queue observations conducted at Cambridgepark Drive at Steel Place on both Tuesday, February, 27, 2018 and Thursday, March 1, 2018.

The queue analysis results presented in the tables above correspond to the level of service analyses conducted for the study area intersections.

[~]Volume exceeds capacity; queue is theoretically infinite.

^{*}SimTraffic results presented instead of Synchro results

¹2018 Observed Average Queues were an average of the trials taken overall on both Tuesday and Thursday.



The queue observations conducted on Tuesday and Thursday, showed similar queue lengths at some Cambridgepark Drive at Steel Place approaches. At the Steel Place southbound approach in the evening on Thursday, approximately 13 more vehicles were observed in queue in the shared lane and approximately 16 more vehicles in the left lane than that on Tuesday. Similarly, Cambridgepark Drive eastbound in the evening on Thursday, had approximately 13 more vehicles in queue than what was observed on Tuesday. The multi-day queue observations indicate that day-to-day operations at Cambridgepark Drive at Steel Place are not always identical and the variation in observed queues has been considered in the traffic analysis.

8 Residential Street Volume Analysis

Roadway segments within the study area with residential street frontage were evaluated to understand Project impacts. The peak hour volumes (both directions) traveling the analyzed roadway segments are presented in Tables 8.a.1 and 8.a.2. For analyzed segments that are between study area intersections, the average volumes at these intersections were taken as the volume traveling along the segment. The analysis shows the percent increase in traffic along the residential roadway segments between Existing and Build volumes and Build and Future volumes.

Of all of the roadway segments in the study area, a total of three of the sixteen segments identified are streets which have more than 1/3 of residential frontage, as determined by the existing first floor use. These segments are evaluated in the Planning Board Criteria for increased volume on residential streets.



TRAFFIC ON STUDY AREA ROADWAYS - MORNING PEAK HOUR **TABLE 8.A.1**

Roadway	Segment	Amount of Residential	Existing ¹	Build	Increase ²	Percent Increase	Future ³	Increase	Percent Increase
	west of 100 Cambridgepark Drive	> 1/3 but <1/2	621	621	0	%0	780	159	76%
	between 100 Cambridgepark Drive and Site West Driveway	1/3 or less	736	799	634	%6	912	113	14%
Cambridgepark Drive	between Site West Driveway and Site East Driveway	1/3 or less	736	799	63	%6	912	113	14%
	between Site East Driveway and Steel Place	1/3 or less	754	799	45	%9	912	113	14%
	between Steel Place and Alewife Brook Parkway	1/3 or less	626	1021	42	4%	1148	127	12%
Steel Place	between Cambridgepark Drive and Alewife Station Access Road	1/3 or less	727	730	8	%0	794	49	%6
	north of Alewife Station Access Road	1/3 or less	1099	1092	-7	-1%	1156	49	%9
Rindge Avenue	west of Alewife Brook Parkway	1/2 or more	948	949	1	%0	986	37	4%
Concord	west of Fresh Pond Rotary	1/3 or less	1765	1778	13	1%	1965	187	11%
Avenue	east of Fresh Pond Rotary	1/3 or less	3550	3568	18	1%	3841	273	%8
	between Fresh Pond Rotary and Rindge Avenue	1/3 or less	3200	3231	31	1%	3418	187	%9
Alewife Brook	between Rindge Avenue and Cambridgepark Drive	1/3 or less	3738	3770	32	1%	3983	213	%9
Parkway	Between Cambridgepark Drive and Route 2/16 Interchange	1/3 or less	3411	3421	10	%0	3569	148	4%
	north of Route 2/16 Interchange	1/3 or less	2344	2356	12	1%	2473	117	2%
Route 2	west of Route 2/16 Interchange	1/3 or less	4251	4259	8	%0	4406	147	3%
Alewife Station Access Road	between Route 2/16 Interchange and Steel Place	1/3 or less	285	295	10	4%	327	32	11%
	1 Where driveways/on-street parking created		flow/outflow volu	me imbalance	a seament inflow/outflow volume imbalance an average was calculated ner direction and added	alculated ner dir	ection and adde	Pa	

Where driveways/on-street parking created a segment inflow/outflow volume imbalance, an average was calculated per direction and added H 2 E 4

Net new project trips after trip credits are applied

Future accounts for area background project volumes, Project generated volumes, and a background growth rate of 0.5%

Not an indication of exact number of net-new project trips; trip credits and project generated trips utilize different driveways (site east and west driveways & 100 Cambridgepark Drive driveway)



TRAFFIC ON STUDY AREA ROADWAYS - EVENING PEAK HOUR **TABLE 8.A.2**

Roadway	Segment	Amount of Residential	Existing ¹	Build	Increase ²	Percent Increase	Future ³	Increase	Percent Increase
	west of 100 Cambridgepark Drive	> 1/3 but <1/2	425	425	0	%0	457	32	%8
	between 100 Cambridgepark Drive and Site West Driveway	1/3 or less	574	639	654	11%	736	26	15%
Cambridgepark Drive	between Site West Driveway and Site East Driveway	1/3 or less	588	639	51	%6	736	97	15%
	between Site East Driveway and Steel Place	1/3 or less	587	637	50	%6	732	95	15%
	between Steel Place and Alewife Brook Parkway	1/3 or less	1261	1307	46	4%	1422	115	%6
Steel Place	between Cambridgepark Drive and Alewife Station Access Road	1/3 or less	799	801	2	%0	858	57	7%
	north of Alewife Station Access Road	1/3 or less	922	924	2	%0	964	40	4%
Rindge Avenue	west of Alewife Brook Parkway	1/2 or more	813	819	9	1%	872	53	%9
Concord	west of Fresh Pond Rotary	1/3 or less	1325	1339	14	1%	1521	182	14%
Avenue	east of Fresh Pond Rotary	1/3 or less	3010	3029	19	1%	3258	229	%8
	between Fresh Pond Rotary and Rindge Avenue	1/3 or less	3091	3124	33	1%	3273	149	2%
Alewife Brook	between Rindge Avenue and Cambridgepark Drive	1/3 or less	3503	3542	39	1%	3757	215	%9
Parkway	Between Cambridgepark Drive and Route 2/16 Interchange	1/3 or less	3180	3187	7	%0	3312	125	4%
	north of Route 2/16 Interchange	1/3 or less	2578	2590	12	%0	2692	102	4%
Route 2	west of Route 2/16 Interchange	1/3 or less	4558	4553	-5	%0	4707	154	3%
Alewife Station Access Road	between Route 2/16 Interchange and Steel Place	1/3 or less	801	801		%0	850	46	%9
	1 Whose district street transfer and 100 comment inflormation of the street of the str	200+001 CP	Coltaino/moltai tac	dai omilow		bobbe bar acitorile sea botel plea sem operative ac	or acitoric	70770	

Where driveways/on-street parking created2578 a segment inflow/outflow volume imbalance, an average was calculated per direction and added 1284

Net new project trips after trip credits are applied

Future accounts for area background project volumes, Project generated volumes, and a background growth rate of 0.5% Not an indication of exact number of net-new project trips due to trip credits and project generated trips utilizing different driveways (site east and west driveways & 100 Cambridgepark Drive driveway)



9 Parking Analysis

9.a Vehicle Parking

According to the City's 1990 parking inventory, the Project site has 68 employee parking spaces registered under 50/54 Cambridgepark Drive for Wyeth Research. As noted above in Section 2.f of this report, a parking utilization study was not conducted for existing buildings on the proposed Project Site. The building and parking lot will be demolished as part of the Project.

The Project is proposing to supply a total of 187 vehicle parking spaces for 299 residential units, at a parking ratio of approximately 0.63 spaces per unit. Residents, on a first come, first serve basis, will have the ability to lease a vehicle parking space with the building's garage. Parking spaces will be managed by the transportation coordinator. Short-term loading and visitors will be accommodated by the new on-street parking along the shared parking as well as limited visitor parking within the proposed garage.

The site is located within walking distance to the Alewife Train Station and several bus routes, which is an indicator of lower parking utilization rates. due to higher numbers of car-free commuters. Table 9.a.1 summarizes other nearby residential building parking rates compared to the proposed Project.

TABLE 9.A.1 AREA RESIDENTIAL BUILDING PARKING RATES

Residence	Total Units (Total Leased Units)	Total Built Parking Spaces (Total Leased Parking Spaces)	Overall Parking Rate (Actual Parking Rate)
Proposed Project	299	187	0.63 spaces/unit
120 Combuideonouk Dul	213	145	0.68 spaces/unit
130 Cambridgepark Dr ¹	(147)	(101)	(0.69 spaces/unit)
160 Combridgements Du?	398	394	0.99 spaces/unit
160 Cambridgepark Dr ²	(369)	(256)	(0.68 spaces/unit)

Source:

Although the nearby 130 and 160 Cambridgepark Drive residential buildings have a leased parking ratio of 0.68 and 0.69 leased spaces/occupied unit, a slightly lower ratio is proposed for the proposed 50 Cambridgepark Drive Project to reflect the fact that the parking occupancy in the other buildings (see Table 9.A.2) is below the actual capacity.

The Project is expected to reflect a pattern of parking occupancy similar to current parking demand for 130 Cambridgepark Drive. 130 Cambridgepark Drive is supported by a garage in the building itself and spaces in the abutting parking garage at 140 Cambridgepark Drive. Garage data for February 2018 was used to estimate how the occupancy of the Project garage

¹ Information provided by Hanover Company as of February 27, 2018

² Information taken from 2017 City of Cambridge TDM Annual Report Summary



would vary throughout the day. The garage data are included in the accompanying CD. Table 9.A.2 presents the estimated average weekday parking occupancy by hour-of-day for the Project based on the parking occupancies for 130 Cambridgepark Drive.

TABLE 9.A.2 ESTIMATED AVERAGE WEEKDAY PROJECT PARKING OCCUPANCY

	130 Cam	bridgepark Drive	Estimated Parking
Time	Number of Occupied Spaces ¹	Ratio of Occupied Spaces per Occupied Unit ²	Occupancy for Proposed Project
12:00-1:00 AM	80	0.55	166
1:00-2:00 AM	80	0.56	167
2:00-3:00 AM	81	0.56	169
3:00-4:00 AM	82	0.57	169
4:00-5:00 AM	82	0.57	170
5:00-6:00 AM	82	0.57	170
6:00-7:00 AM	81	0.56	167
7:00-8:00 AM	76	0.52	157
8:00-9:00 AM	65	0.45	135
9:00-10:00 AM	55	0.38	113
10:00-11:00 AM	48	0.33	99
11:00-12:00 PM	45	0.31	92
12:00-1:00 PM	43	0.30	89
1:00-2:00 PM	42	0.29	87
2:00-3:00 PM	41	0.29	86
3:00-4:00 PM	40	0.28	83
4:00-5:00 PM	42	0.29	87
5:00-6:00 PM	47	0.33	99
6:00-7:00 PM	54	0.38	113
7:00-8:00 PM	61	0.43	128
8:00-9:00 PM	57	0.40	119
9:00-10:00 PM	71	0.49	147
10:00-11:00 PM	75	0.52	156
11:00-12:00 PM	78	0.54	161

¹ Based on dedicated residential parking space data for February, 2018 in 130 Cambridgepark Drive garage and abutting garage at 140 Cambridgepark Drive, provided by Hanover Company.

As shown, peak parking demand on an average weekday is estimated at 0.57 spaces per occupied unit, occurring over-night as expected. Therefore, the peak parking occupancy on an average weekday for the 299 units in the proposed Project is 170 spaces out of the total 187 spaces proposed. However, review of the parking data for 130 Cambridgepark Drive shows that the absolute maximum over-night demand on any single day was as high as 0.66 spaces per occupied unit. This absolute maximum occurs only once in the entire month of February.

² Number of occupied parking spaces per 144 total occupied residential unit at 130 Cambridgepark Drive. [where 144 was the average # of occupied units for the month of February 2018]



Otherwise, over-night parking rates reach as high as 0.62, therefore the 187 parking spaces at a parking rate of 0.63 spaces/unit for 50 Cambridgepark Drive is expected to be adequate to meet the 50 Cambridgepark Drive's expected parking demand.

9.a Bicycle Parking

The Project will provide parking in accordance with the City of Cambridge's Bicycle Parking Zoning Ordinance, as shown in Table 9.b.1.

TABLE 9.B.1 BICYCLE PARKING

Type of Parking	Parking Rate	# of Bicycle Spaces	
Residential			
Long Term	1.05 spaces per dwelling ¹	313	
Short Term	0.10 spaces per dwelling	30	
	Total	343	
Retail/Restaurant			
Long Term	0.2 spaces per 1,000 SF	2	
Short Term	1.00 spaces per 1,000 SF	8	
	Total	9	
Total Long Term		315	
Total Short Term		38	

Source: City of Cambridge Zoning Ordinance Article 6.100

The Project will provide 315 long term bicycle parking spaces in two ground level bike rooms within the building. Each room will have direct access to the building exterior and sidewalk. The Project will also provide 37 short term spaces located along Cambridgepark Drive to support visitors and patrons to the site. The total 352 spaces will result in the installation of approximately 176 bicycle racks (assuming each rack fits 2 bicycles). Although the type of bike racks have not been selected, they will be similar, if not the same, as those bike racks installed at 88 and 130 Cambridgepark Drive Residences.

Figures G.1 - G.3 presented above illustrate the location and layout of the long term and short term bicycle parking spaces.

10 Transit Analysis

As requested by the City's Scoping Letter, a transit analysis has been conducted for the Project. The analysis reviewed existing Red Line operations and assessed the impacts of project-generated transit trips and future transit trips.

The following sections summarize existing transit services availability in the study area and provide an assessment of transit utilization and capacity for the key transit line, the Red Line,

¹ per city guide – 1.00 spaces per unit for the first 20 units for a residential building



accessed at Alewife Station. Although several bus routes are also accessed by Alewife Station, the transit analysis assumes transit rider trips produced by the background projects and the Proposed project will all be Red Line rider trips.

The transit analysis was based on the following 8-step methodology:

- 1. Quantify the existing transit system capacity
- 2. Quantify the existing system ridership
- 3. Report on existing transit system utilization (ridership/capacity) 2018 Existing Conditions
- 4. Develop and assign project-generated transit trips to the existing transit system
- 5. Report on project impacts to the transit system utilization 2018 Build Conditions
- 6. Grow 2018 existing transit system ridership to year 2023
- 7. Compile area background project transit trips and assign to transit system network
- 8. Report on future transit system utilization (impacts from project as well as other background projects and general system growth) 2023 Future Conditions

The V/C ratio (Volume to Capacity) is the resulting metric that is used to reflect the level of utilization for each transit service line. The V/C ratios (or utilization rates) are presented for the Existing Condition (2016), Build Condition (Existing + Project trips), and Future Condition (Existing + Project trips + background growth).

10.a Existing Transit System Capacity – STEP 1

The capacity of a transit line depends on the number of trains operating during a specified time period (frequency), the number of people that can be accommodated on a vehicle (a train car), and the number of individual cars in each train.

The study period for this analysis includes the morning and evening transit peak hours defined as 8:00 AM to 9:00 AM and 5:00 PM to 6:00 PM, respectively.

Train frequencies were compiled from latest published MBTA schedules¹ and MBTA Bus Ridecheck data from Winter 2018, as presented in Table 10.a.1.

The vehicle load standards (i.e. number of people safely and comfortably riding on a train car) are based on the MBTA's Service Delivery Policy² and the MBTA *Blue Book* (14th Edition) data (Red Line policy capacity of 167 passengers per car, with a standard operation of 6-car trains).

The average Red Line on-time performance was adjusted by 89%, based on the 30-day average (February 12 to March 14, 2018) provided by the MBTA Dashboard. The on-time performance adjustment of 89% reduced the number of available trains during peak hour to account for schedule irregularities and resulting wait times experienced by the passengers.

¹ MBTA schedules, Winter 2018

² MBTA Service Delivery Policy, approved by the Board of Directors in June 2010



Table 10.a.1 shows the resulting system capacities for the Red Line based on MBTA provided data.

TABLE 10.A.1 SYSTEM PEAK HOUR CAPACITY (PER MBTA DATA)

Mode	Frequency ^(a)	OTP Factor ^(b)	# Passengers / Vehicle ^(c)	# Cars / Train	Resulting Capacity ^(d) (# Passengers per Peak Hour)		
Red Line at Alewife Station							
Inbound	13	0.89	167	6	11,593		
Outbound	13	0.89	167	6	11,593		

Notes:

- (a) Number of vehicles per hour, per MBTA published schedules
- (b) On-Time Performance Factor from MBTA Dashboard as of March 15, 2018
- (c) Number of policy level capacity per MBTA Blue Book 14th Edition
- (d) Calculated Capacity = # of Trains x OTP Factor x # pax per vehicle x # of cars shown as number of passengers per peak hour

252 new Red line cars are scheduled to be delivered between 2019-2023 along with improvements in signal equipment which will significantly increase capacity and address overcrowding at some stations along the Red Line. MBTA Red / Orange Line New Vehicle Technical Provisions (May 2014) report indicates that capacity increase will allow a decrease in the existing headway from 4.5 minutes to 3 minutes for an approximately additional 7,000 transit riders per hour.

Table 10.a.2 shows the resulting system capacities for the Red Line based on MBTA provided data and technical provisions. Steps 6 and 7 are performed considering both existing Red line capacity as well as this future condition.

TABLE 10.A.2 FUTURE SYSTEM PEAK HOUR CAPACITY (PER MBTA DATA)

Mode	Frequency ^(a)	OTP Factor ^(b)	# Passengers / Vehicle ^(c)	# Cars / Train	Resulting Capacity ^(d) (# Passengers / Peak Hour)			
Red Line at Alewife Station								
Inbound	20	0.89	175	6	18,690			
Outbound	20	0.89	175	6	18,690			

Notes:

- (e) Number of vehicles per hour, per MBTA presentation to the Fiscal & Management Control Board (September 19, 2016)
- (f) On-Time Performance Factor from MBTA Dashboard as of March 15, 2018
- (g) MBTA technical provisions:
 - 280 avg. pax/car (published crush capacity) No available published policy capacity so existing crush-to-policy ratio of 1.6 used to estimate future policy capacity
- (h) Calculated Capacity = # of Trains x OTP Factor x # pax per vehicle x # of cars shown as number of passengers per peak hour



10.b Existing Transit System Ridership and Utilization – Steps 2 & 3

The MBTA Ridership data from Fall 2016 was used to obtain peak hour passenger loads. Red Line ridership for the analysis was based on data for Alewife Station from Fall 2016 with no growth rate adjustments applied (consistent with vehicle growth assumption).

Inbound trains start their trip from Alewife Station and continue to Ashmont or Braintree Stations, and Outbound trains end at Alewife Station from either Ashmont or Braintree Stations. Since this is the end of the Red Line, passengers board the inbound line and exit the outbound line. Specific boarding and alighting volumes during the morning and evening peak hours are presented in the accompanying CD.

Combining the system capacity developed in Step 1 and the system ridership, the system's utilization rates were calculated and are presented in Table 10.b.1.

TABLE 10.B.1 EXISTING TRANSIT SERVICE UTILIZATION (PER MBTA DATA)

Route and Direction	Capacity	pacity Morning Peak Evenin Hour Ridership Hour Ri		Morning Peak Hour V/C	Evening Peak Hour V/C
Red Line at Alewife Station					
Inbound Exiting Alewife	11,593	2,502	917	0.22	0.08
Outbound Entering Alewife	11,593	631	2,316	0.05	0.20

As shown in Table 10.b.1, the existing Red Line at Alewife Station is operating with V/C ratios below 1.0 in the morning and evening inbound and outbound directions.

10.c Development of Transit Project Trips – Step 4

As presented in Section 3 of this report, the Project is expected to generate 101 transit trips (25 entering, 76 exiting) during the morning peak hour and 104 transit trips (72 entering, 32 exiting) during the evening peak hour, according to the trip generation calculations. For a conservative analysis, no transit trip credits were taken into account for the existing office building.

As discussed above, project transit trips were all assigned to the Red line. A detailed transit distribution by direction and peak hour is presented in Table 10.c.1.



TABLE 10.C.1 TRANSIT TRIP DISTRIBUTION

	Morning F	Peak Hour	Evening Peak Hour						
	% OUT % IN		% OUT	% IN					
Red Line at Alewife Station									
Inbound	100%	0.0%	100%	0.0%					
Outbound	0.0%	100%	0.0%	100%					
Total	100%	100%	100%	100%					

Source: MBTA existing station ridership levels, Fall 2016

Transit distribution is then applied to the Project-generated transit trips in order to determine the Project-generated transit trips, as presented in Table 10.c.2.

TABLE 10.C.2 PROJECT-GENERATED TRANSIT TRIPS BY LINE

Route and	Мог	ning Peak Hou	r	Eve	Evening Peak Hour						
Direction	Trips OUT (Boardings)	Trips IN (Alightings)	Trips Total	Trips OUT (Boardings)	Trips IN (Alightings)	Trips Total					
Red Line at Alewife Station											
Inbound	76	0	76	32	0	32					
Outbound	0	25	25	0	72	72					
Total	76	25	101	32	72	104					

10.d Build Transit System Utilization – Step 5

The Project-generated transit trips from Step 4 above are added to the existing route volumes to develop the "Build Condition" utilization scenario (Existing + Project trips). Resulting v/c ratios are presented in Table 10.d.1.

TABLE 10.D.1 BUILD CONDITION TRANSIT SERVICE UTILIZATION (PER MBTA DATA)

Route and Direction	Morning I Capacity Hour Ride Route and Direction Policy (Existing (from Step Project To 1)		Evening Peak Hour Ridership (Existing + Project Trips)	Morning Peak Hour V/C	Evening Peak Hour V/C
Red Line at Alewife Station					
Inbound Exiting Alewife	11,593	2,578	949	0.22	0.08
Outbound Entering Alewife	11,593	656	2,388	0.06	0.21



As presented in Table 10.d.1 and compared to Table 10.b.1, the Red Line is expected to operate at similar levels in the Build Condition as under Existing Conditions with only minor increases, if any, in the V/C ratios.

10.e Development of Future Transit Trips – Step 6

To analyze the 2023 Future Condition for transit, the MBTA existing ridership was grown to year 2023 based on growth rates presented in the July 2015 MIT Kendall Square TIS (4% per year assumption for the Red Line ridership). As mentioned above, the Fall 2016 MBTA Ridership data was assumed to be the baseline 2018 existing condition with no growth rate adjustments applied. This 2018 baseline data was then grown by 4% per year for 5 years to derive the future baseline ridership. The project generated transit trips, presented in Table 10.c.2, were then added to the ridership estimates. The 2023 Future ridership is presented in Table 10.e.1.

TABLE 10.E.1 2023 FUTURE GROWTH TRANSIT SERVICE UTILIZATION (PER MBTA DATA)

	Capacity Policy	Morning Peak Hour Ridership	Evening Peak Hour Ridership	Morning Peak Hour V/C	Evening Peak Hour V/C
Red Line at Alewife Station (ba	ased on Exis	sting Capacity)			
Inbound Exiting Alewife	11,593	3,121	1,148	0.27	0.10
Outbound Entering Alewife	11,593	793	2,890	0.07	0.25
Red Line at Alewife Station (ba	ased on Fut	ure Capacity)			
Inbound Exiting Alewife	18,690	3,121	1,148	0.17	0.06
Outbound Entering Alewife	18,690	793	2,890	0.04	0.15

As presented in Table 10.e.1, because of the scheduled improvements, the Red Line is expected to operate in the Build Condition with V/C ratios better than under existing conditions.

10.f Compile and Assign Area Background Project Transit Trips – Step 7

In addition to growing the transit trips to 2023 Future Conditions, it is necessary to add transit trips from area projects that have not yet come on-line. The same projects listed in the traffic analysis were also used in this transit analysis. Transit trips for each background project, as presented in Table 10.f.1 below, were included in the Future analysis.



TABLE 10.F.1 BACKGROUND PROJECT TRANSIT TRIPS

During -	Моі	ning Peak	Hour	Evening Peak Hour			
Project	In	Out	Total	In	Out	Total	
605 Concord Ave	2	7	9	14	7	21	
87-95 Fawcett Street	2	7	9	7	4	11	
75 New Street	3	12	15	12	6	18	
130 Cambridgepark Drive	9	36	45	35	19	54	
88 Cambridgepark Drive	20	89	109	109	59	168	
35 Cambridgepark Drive renovation project	13	2	15	5	13	18	
55 Wheeler Street	15	62	77	61	33	94	
The Residences at Alewife Station (195 & 211 Concord Turnpike)	28	67	95	38	38	76	
TOTAL	92	282	374	184	126	310	

Similar to the Project generated transit trips, all of the background transit trips were assigned to the Red Line.

10.g Future Transit System Utilization – Step 8

The 2023 Future transit scenario is based on grown ridership levels, combined with background project transit trips and Project-generated transit trips. The resulting transit ridership and calculated V/C ratios for morning and evening peak hours for 2023 Future Condition is shown in Table 10.q.1.

TABLE 10.G.1 2023 FUTURE GROWTH CONDITION WITH BACKGROUND PROJECTS TRANSIT SERVICE UTILIZATION

Route and Direction	Capacity Policy (from Step 1)	Morning Peak Hour Ridership (2023 Future + Background Project Trips)	Evening Peak Hour Ridership (2023 Future + Background Project Trips)	Morning Peak Hour V/C (a)	Evening Peak Hour V/C (a)
Red Line at Alewife Station (ba	sed on Existing	g Capacity)			
Inbound Exiting Alewife	11,593	3,403	1,327	0.29	0.11
Outbound Entering Alewife	11,593	885	3,171	0.08	0.27
Red Line at Alewife Station (ba	sed on Future	Capacity)			
Inbound Exiting Alewife	18,690	3,403	1,327	0.18	0.07
Outbound Entering Alewife	18,690	885	3,171	0.05	0.17



As presented in Table 10.g.1, the Red Line is again expected to operate in the Future Condition with V/C ratios better than under Existing conditions.

11 Pedestrian Analysis

Pedestrian crossing volumes at study area intersections are presented above in Figure 2.c.4. The results of pedestrian level of service (PLOS) analysis at intersection crosswalks are presented in Table 11.a.1 for signalized intersections and Table 11.a.2 for unsignalized intersections, as well as graphically illustrated in Figures 11.a.1 and 11.a.2.

Pedestrian level of service at signalized intersections is dictated by the portion of the signal cycle dedicated to the pedestrian crossings. Accordingly, increasing pedestrian volumes does not alter pedestrian level of service at signalized intersections, and no changes in PLOS are projected under build or future conditions. It is assumed that the walk time and cycle length at these intersections will not change from existing and therefore PLOS will remain consistent.

For unsignalized intersections, the PLOS is calculated using the crosswalk length and the conflicting vehicle floor rates for morning and evening peak hours.

The only intersection that shows a slight change in PLOS with the addition of Project trips is Cambridgepark Drive at the Shared Driveway. The intersection crosswalk on the south side of Cambridgepark Drive changes from A to B in the morning peak hour. This change occurs due to the removal of the two existing site driveways from the Project Site elimination of those associated pedestrian-vehicle conflicts, and the utilization of the existing driveway between 100 Cambridgepark Drive and the Project Site as the proposed Project access. The change includes the addition of 65 net new vehicles trips (44 entering and 21 exiting) that conflict with pedestrian movement as the vehicles pass through the crosswalk. The impact is minimal, with the addition of 2.38 seconds added during the morning peak which barely tips the LOS threshold at this crosswalk location. All other intersections show no change in PLOS with the addition of project trips. Figures 11.a.1 and 11.a.2 show the PLOS for the various conditions for morning and evening peak hour.



TABLE 11.A.1 SIGNALIZED INTERSECTION – PEDESTRIAN LOS SUMMARY

		Mori	ning Peak H	lour	Evening Peak Hour			
Intersection	Crosswalk	Existing 2018	Build 2018	Future 2023	Existing 2018	Build 2018	Future 2023	
	East	D	D	D	Е	Е	E	
Cambridgepark Drive/Steel	West	D	D	D	Е	Е	E	
Place	North	D	D	D	Е	Е	E	
	South	D	D	D	Е	Е	Е	
Cambridgepark Drive/Alewife Brook Parkway		1	No pedestria	an facilities	provided			
Alewife Brook	East	E	E	E	E	E	E	
Parkway/Rindge Avenue	South	E	Е	Е	E	Е	E	
Alewife Brook Parkway at Route 2/16	East	E	E	Е	E	E	E	

TABLE 11.A.2 UNSIGNALIZED INTERSECTION – PEDESTRIAN LOS SUMMARY

		Mor	ning Peak H	lour	Evening Peak Hour			
Intersection	Crosswalk	Existing 2018	Build 2018	Future 2023	Existing 2018	Build 2018	Future 2023	
Cambridgepark Drive/100 Cambridgepark Drive Driveway	South	Α	В	С	В	В	С	
Cambridgepark Drive/Site West (outbound) Driveway	South	Α	*	*	Α	*	*	
Cambridgepark Drive/Site	West	F	*	*	Е	*	*	
East (inbound) Driveway	South	Α	*	*	Α	*	*	
Steel Place/Alewife Station	East	В	В	С	Е	Е	E	
Access Road (Route 2	West	Α	Α	В	Α	Α	Α	
Connector)	North	F	F	F	E	Е	E	



12 Bicycle Analysis

12.a Conflicting Movements

Conflicting vehicle turning movements at the study area intersections are presented above in Figure 2.c.1 and 2.c.2, and summarized in Table 12.a.1 for Existing 2018, Build 2018, and Future 2023 conditions.

 TABLE 12.A.1
 CONFLICTING BICYCLE/VEHICLE MOVEMENTS AT STUDY INTERSECTIONS

			Existing		Conflic	ting Veh	icle Mov	ements	
			Peak Hour	Existin	g 2018	Build	2018	Future	2023
Intersection	Time Period	Bicycle Direction	Bicycle Volume	Right Turna	Left Turn ^b	Right Turna	Left Turn ^b	Right Turn ^a	Left Turn ^b
	Morning	EB	3	5	104	5	121	5	139
		WB	3	n/a	n/a	n/a	n/a	n/a	n/a
Cambridgepark		NB	3	23	n/a	69	n/a	144	n/a
Drive/100 Cambridgepark Drive	Evening	EB	2	6	31	6	75	6	109
Cambridgepark Drive		WB	1	n/a	n/a	n/a	n/a	n/a	n/a
		NB	0	132	n/a	153	n/a	184	n/a
	Morning	EB	5	0	0	-	-	-	-
		WB	2	n/a	n/a	-	-	-	-
Cambridgepark		NB	0	0	n/a	-	-	-	-
Drive/Site West (outbound) Driveway	Evening	EB	2	0	0	-	-	-	-
(outbound) Driveway		WB	1	n/a	n/a	-	-	-	-
		NB	3	14	n/a	-	-	-	-
	Morning	EB	5	0	18	-	-	-	-
Cambridgepark Drive/Site East		WB	2	n/a	n/a	-	-	-	-
(inbound) Driveway	Evening	EB	3	2	1	-	-	-	-
(WB	1	n/a	n/a	-	-	-	-
	Morning	EB	5	5	23	5	23	5	24
		WB	3	120	19	120	29	139	44
		NB	2	48	208	48	208	49	224
Cambridgepark		SB	2	147	3	140	3	147	3
Drive/Steel Place	Evening	EB	2	6	19	6	19	6	19
		WB	2	69	37	69	37	77	44
		NB	2	93	576	93	576	95	601
		SB	1	37	3	39	3	48	3



			Existing	Conflicting Vehicle Movements						
			Peak Hour	Existin	g 2018	Build	2018	Future	e 2023	
Intersection	Time Period	Bicycle Direction	Bicycle Volume	Right Turna	Left Turn ^b	Right Turna	Left Turn ^b	Right Turna	Left Turn ^b	
	Morning	EB	2	278	n/a	304	n/a	365	n/a	
		NB	0	n/a	n/a	n/a	n/a	n/a	n/a	
Cambridgepark Drive/Alewife Brook		SB	1	298	252	298	258	313	289	
Parkway	Evening	EB	0	544	n/a	552	n/a	593	n/a	
· aay		NB	0	n/a	n/a	n/a	n/a	n/a	n/a	
		SB	0	98	171	106	202	115	251	
	Morning	WB	0	595	n/a	596	n/a	624	n/a	
		NB	0	143	n/a	143	n/a	147	n/a	
Alewife Brook Parkway/Rindge		SB	0	n/a	n/a	n/a	n/a	n/a	n/a	
Avenue	Evening	WB	0	455	n/a	461	n/a	505	n/a	
		NB	0	195	n/a	195	n/a	200	n/a	
		SB	0	n/a	n/a	n/a	n/a	n/a	n/a	
	Morning	NB	0	254	83	264	83	289	92	
Steel Place/Alewife Station Access Road		SB	0	348	n/a	348	n/a	357	n/a	
(Route 2 Connector)	Evening	NB	0	363	403	363	403	400	414	
		SB	0	34	n/a	34	n/a	35	n/a	
	Morning	EB^1	0	n/a	n/a	n/a	n/a	n/a	n/a	
		WB	0	45	n/a	45	n/a	46	n/a	
		SB	7	245	n/a	255	n/a	271	n/a	
Fresh Dond Potany		SWB	1	5	n/a	5	n/a	5	n/a	
Fresh Pond Rotary	Evening	EB^1	0	n/a	n/a	n/a	n/a	n/a	n/a	
		WB	0	10	n/a	10	n/a	10	n/a	
		SB	0	280	n/a	284	n/a	311	n/a	
		SWB	0	0	n/a	0	n/a	0	n/a	
	Morning	WB	2	n/a	n/a	n/a	n/a	n/a	n/a	
Alewife Brook		SB	0	n/a	n/a	n/a	n/a	n/a	n/a	
Parkway at Route 2/16 – Signal A	Evening	WB	0	n/a	n/a	n/a	n/a	n/a	n/a	
2/10 – Signal A	3	SB	0	n/a	n/a	n/a	n/a	n/a	n/a	
	Morning	EB	0	n/a	n/a	n/a	n/a	n/a	n/a	
		WB	0	n/a	n/a					
		VVD	U	II/a	11/a	n/a	n/a	n/a	n/a	
		NB	2	n/a	n/a	n/a	n/a	n/a	n/a	
Alewife Brook Parkway at Route		SB	1	n/a	n/a	n/a	n/a	n/a	n/a	
2/16 – Signal B	Evening	EB	0	n/a	n/a	n/a	n/a	n/a	n/a	
		WB	0	n/a	n/a	n/a	n/a	n/a	n/a	
		NB	0	n/a	n/a	n/a	n/a	n/a	n/a	
		SB	1	n/a	n/a	n/a	n/a	n/a	n/a	



			Existing	Conflicting Vehicle Movements							
			Peak Hour	Existin	g 2018	Build	2018	Future	e 2023		
Intersection	Time Period	Bicycle Direction	Bicycle Volume	Right Turna	Left Turn ^b	Right Turna	Left Turn ^b	Right Turn ^a	Left Turn ^b		
	Morning	WB	0	56	n/a	62	n/a	83	n/a		
Alewife Brook		NB	0	n/a	n/a	n/a	n/a	n/a	n/a		
Parkway at Route 2/16 – Signal C	Evening	WB	0	303	n/a	304	n/a	325	n/a		
2,10 3igilai e		NB	0	n/a	n/a	n/a	n/a	n/a	n/a		
	Morning	EB	0	n/a	n/a	n/a	n/a	n/a	n/a		
Alewife Brook		SB	1	n/a	n/a	n/a	n/a	n/a	n/a		
Parkway at Route 2/16 – Signal D	Evening	EB	0	n/a	n/a	n/a	n/a	n/a	n/a		
2, 10 0.gridi D		SB	1	n/a	n/a	n/a	n/a	n/a	n/a		

a Advancing volume

13 Transportation Demand Management

The Proponent will implement a program of transportation demand management (TDM) actions to reduce automobile trips generated by the Project. The goal of the Project's TDM plan is to reduce the use of single occupant vehicles (SOVs) by encouraging carpooling and vanpooling, bicycle commuting and walking, and increased use of the area's public transportation system by residents.

The Proponent will consider the following TDM programs as part of the proposed Project to encourage residents to use alternatives to SOV travel:

- Make available 1 carshare parking space for a vehicle-sharing company.
- Subsidize MBTA passes for new building residents.
- Encourage car/vanpooling in coordination with MassRIDES or other private ridematching service provider.
- Provide up to 4 EV-ready parking spaces.
- Provide air pumps and other bike tools, such as a "fix-it" stand in the bicycle storage areas.
- Do not charge residents additional fees for regular bicycle parking.
- Charge parking separately from the residential rent.
- Join the Alewife Transportation Management Association (TMA).
- Designate a transportation coordinator (TC) for the site to manage the TDM program.
- Post information in a prominent location in the building and on the building's website, social media and property newsletters promoting the use of transportation options and service information.
- Provide packages for new residents providing information on transit and other alternative transportation modes.

b Opposing volume

NA Movement not available

Bicycle path is independent from the roadway



14 Transportation Mitigation

The proposed Project exceeds 14 out of 143 possible data entries, resulting in an 8% exceedance rate. As requested by the TP&T Department, Table 13.a.1 provides a listing of all Planning Board Special Permit Exceedances, and indicates how transportation mitigation measures will or cannot mitigate the Project Exceedances.

TABLE 14.A.1 EXCEEDANCE MITIGATION SUMMARY

#	Location		Reason for Exceedance	Mitigation
		Criteria E-1 Pedes	trian Delay	
1 2 3 4	Cambridgepark Drive at Steel Place	East Crosswalk – Evening West Crosswalk – Evening North Crosswalk – Evening South Crosswalk - Evening	Existing and Build PLOS = E. Threshold is PLOS D with the project	Existing PLOS conditions are maintained at this location with the construction of the Project and do not deteriorate in the Build Condition.
5 6 7 8	Alewife Brook Parkway at Rindge Avenue	East Crosswalk – Morning East Crosswalk – Evening South Crosswalk - Morning South Crosswalk - Evening	Existing and Build LOS = E. Threshold is PLOS D with the project.	Existing PLOS conditions are maintained at this location with the construction of the Project and do not deteriorate in the Build Condition.
9	Alewife Brook Parkway at Route 2/16	East Crosswalk – Morning East Crosswalk – Evening	Existing and Build LOS = E. Threshold is PLOS D with the project.	Existing PLOS conditions are maintained at this location with the construction of the Project and do not deteriorate in the Build Condition.
11	Cambridgepark Drive at 100 Cambridgepark Drive Driveway	South Crosswalk – Morning	Existing PLOS = A and Build PLOS = B. Threshold is PLOS A with project.	Project consolidates 3 curb cuts into 1. No mitigation proposed.
12 13 14	Steel Place at Alewife Station Access Road (Route 2 Connector)	East Crosswalk - Evening North Crosswalk – Morning North Crosswalk – Evening	Existing and Build LOS = E and F. Threshold is PLOS D with the project.	Existing PLOS conditions are maintained at this location with the construction of the Project and do not deteriorate in the Build Condition.



Planning Board Special Permit Criteria

Criterion A – Project Vehicle Trip Generation

Table A-1 presents the Project vehicle trip generation criterion. Project vehicle trip generation is based on ITE trip rates, adjusted for local mode split and vehicle occupancy rates as discussed above.

TABLE A-1 PROJECT VEHICLE TRIP GENERATION

Time Period	Criteria (trips)	Build*	Exceeds Criteria?
Weekday Daily	2,000	602	No
Weekday Morning Peak Hour	240	63	No
Weekday Evening Peak Hour	240	65	No

^{*} Does not include trips eliminated by elimination of existing site use

The Project is not expected to exceed the Planning Board Criteria for daily, morning peak, and evening peak Project vehicle trip generation under the Build program.

Criterion B – Vehicle LOS

The criteria for a Project's impact to traffic operations at signalized intersections are summarized in Table B-1 below. These criteria are evaluated for each signalized study-area intersection and presented in Table B-2.

TABLE B-1 CRITERION - VEHICULAR LEVEL OF SERVICE

Existing	With Project
VLOS A	VLOS C
VLOS B, C	VLOS D
VLOS D	VLOS D or 7% roadway volume increase
VLOS E	7% roadway volume increase
VLOS F	5% roadway volume increase



TABLE B-2 VEHICULAR LEVEL OF SERVICE

		Morning	Peak Hour			Evening Peak Hour				
Intersection	Existing Condition	Build Condition	Traffic Increase	Exceeds Criterion?	Existing Condition	Build Condition	Traffic Increase	Exceeds Criterion?		
Cambridgepark Drive/100 Cambridgepark Drive Driveway	С	С	9%	No	В	В	11%	No		
Cambridgepark Drive/Site West (outbound) Driveway	А	-	9%	No	В	-	9%	No		
Cambridgepark Drive/Site East (inbound) Driveway	А	-	6%	No	А	-	9%	No		
Cambridgepark Drive/Steel Place	С	С	4%	No	С	С	3%	No		
Cambridgepark Drive/Alewife Brook Parkway	E	E	1%	No	F	F	1%	No		
Alewife Brook Parkway/Rindge Avenue	F	F	1%	No	F	F	1%	No		
Steel Place/Alewife Station Access Road (Route 2 Connector)	F	F	0%	No	F	F	0%	No		
Fresh Pond Rotary	F	F	1%	No	F	F	1%	No		
Alewife Brook Parkway at Route 2/16 – Signal A	В	В	0%	No	В	В	0%	No		
Alewife Brook Parkway at Route 2/16 – Signal B	E	E	0%	No	F	F	0%	No		
Alewife Brook Parkway at Route 2/16 – Signal C	С	С	3%	No	В	В	0%	No		
Alewife Brook Parkway at Route 2/16 – Signal D	В	В	0%	No	А	А	1%	No		



Criterion C – Traffic on Residential Streets

This criterion considers the magnitude of Project vehicle trip generation during any peak hour that may reasonably be expected to arrive and/or depart by traveling on a residential street. The criteria, based on a Project-induced traffic volume increase on any two-block residential street segment in the study area, are summarized in Table C-1.

TABLE C-1 CRITERION – TRAFFIC ON RESIDENTIAL STREETS

Parameter 1: Amount	Parameter 2: Current Peak Hour Street Volume (two-way vehicles)						
of Residential ¹	< 150 VPH	150-400 VPH	> 400 VPH				
1/2 or more	20 VPH ²	30 VPH ²	40 VPH ²				
>1/3 but <1/2	30 VPH ²	45 VPH ²	60 VPH ²				
1/3 or less	No Max.	No Max.	No Max				

^{1 -} Amount of residential for a two block segment as determined by first floor frontage

VPH - Vehicles per hour

15 of the 23 roadway segments in the study area identified as street segments which have more than 1/3 of residential frontage, and are therefore evaluated against the traffic volume criteria. The results are presented in Table C-2.

TABLE C-2 TRAFFIC ON RESIDENTIAL STREETS

			Moi	rning Peak H	lour	Eve	ning Peak H	our
Roadway	Segment	Amount of Residential	Existing ¹	Increase ²	Exceeds Criteria?	Existing ¹	Increase ²	Exceeds Criteria?
	west of 100 Cambridgepark Drive	> 1/3 but <1/2	621	0	No	425	0	No
	between 100 Cambridgepark Drive and Site West Driveway	1/3 or less	736	63	No	574	65	No
Cambridgepark Drive	between Site West Driveway and Site East Driveway	1/3 or less	736	63	No	588	51	No
	between Site East Driveway and Steel Place	1/3 or less	754	45	No	587	50	No
	between Steel Place and Alewife Brook Parkway	1/3 or less	979	42	No	1261	46	No
Steel Place	between Cambridgepark Drive and Alewife Station Access Road	1/3 or less	727	3	No	799	2	No

^{2 -} Additional Project vehicle trip generation in vehicles per lane, both directions



			Мо	rning Peak H	lour	Eve	ning Peak H	our
Roadway	Segment	Amount of Residential	Existing ¹	Increase ²	Exceeds Criteria?	Existing ¹	Increase ²	Exceeds Criteria?
	north of Alewife Station Access Road	1/3 or less	1099	-7	No	922	2	No
Rindge Avenue	west of Cambridgepark Drive	1/2 or more	948	1	No	813	6	No
Concord	west of Fresh Pond Rotary	1/3 or less	1765	13	No	1325	14	No
Avenue	east of Fresh Pond Rotary	1/3 or less	3550	18	No	3010	19	No
	between Fresh Pond Rotary and Rindge Avenue	1/3 or less	3200	31	No	3091	33	No
Alewife Brook	between Rindge Avenue and Cambridgepark Drive	1/3 or less	3738	32	No	3503	39	No
Parkway	Between Cambridgepark Drive and Route 2/16 Interchange	1/3 or less	3411	10	No	3180	7	No
	north of Route 2/16 Interchange	1/3 or less	2344	12	No	2578	12	No
Route 2	west of Route 2/16 Interchange	1/3 or less	4251	8	No	4558	-5	No
Alewife Station Access Road	between Route 2/16 Interchange and Steel Place	1/3 or less	285	10	No	801	0	No

Note: Volume interpolated from nearest data available in study area

Criterion D – Lane Queue

The criteria for a project's impact to queues at signalized intersections are summarized in Table D-1 below. These criteria are evaluated for each lane group at study-area signalized intersections and presented in Table D-2.

TABLE D-1 CRITERION - VEHICULAR QUEUES AT SIGNALIZED INTERSECTIONS

Existing	With Project
Under 15 vehicles	Under 15 vehicles, or 15+ vehicles with an increase of 6 vehicles
15 or more vehicles	Increase of 6 vehicles

¹ Where driveways/on-street parking created a segment inflow/outflow volume imbalance, an average was calculated per direction and added

² Net new project trips after trip credits are applied



TABLE D-2 LENGTH OF VEHICULAR QUEUES AT SIGNALIZED INTERSECTIONS

		Morr	ning Peak	Hour	Evening Peak Hour			
Intersection	Lane	2018 Existing	2018 Build	Exceeds Criteria?	2018 Existing	2018 Build	Exceeds Criteria?	
	Steel Place NB L/T/R	1	1	No	1	1	No	
	Steel Place SB L	4	4	No	7	7	No	
Cambridgepark	Steel Place SB L/T/R	1	1	No	7	7	No	
Drive/Steel Place	Cambridgepark Drive EB L/T/R	4	5	No	8	8	No	
	Cambridgepark Drive WB L/T	10	11	No	4	5	No	
	Cambridgepark Drive WB R	0	0	No	0	0	No	
	Alewife Brook Parkway NB L	4*	5*	No	4*	4*	No	
Cambridgepark	Alewife Brook Parkway NB T	5*	5*	No	6*	6*	No	
Drive/Alewife Brook Parkway	Alewife Brook Parkway SB T	~39	~39	No	~23	~29	No	
	Cambridgepark Drive EB	3	3	No	8*	9*	No	
Alewife Brook Parkway/Rindge Avenue	Alewife Brook Parkway NB	63*	63*	No	91*	91*	No	
Parkway/Rindge Avenue	Alewife Brook Parkway SB	7*	4*	No	7*	7*	No	
	Rindge Avenue WB L	7	7	No	7*	7*	No	
	Rindge Avenue WB R	~18	~19	No	27*	27*	No	
	Alewife Brook Parkway NB L	~25	~26	No	~24	~24	No	
	Alewife Brook Parkway NB T	4	4	No	3	3	No	
	Alewife Brook Parkway SB T	7	7	No	4	4	No	
	Alewife Brook Parkway SB R	17	17	No	15	15	No	
Alewife Brook Parkway	Route 2 EB L	~11	~11	No	~11	~11	No	
at Route 2/16	Route 2 EB R	9	9	No	6	6	No	
	Alewife Station Exit Ramp WB	3	2	No	7	7	No	
	Alewife Station Exit Ramp WB	1	1	No	3	3	No	

Note: Synchro provides queue data in feet, the table presents queue data in number of vehicles. As directed by the TIS guidelines 1 vehicle = 25 ft

Criterion E – Pedestrian and Bicycle Facilities

Criteria 1: Pedestrian Delay

Pedestrian delay is a measure of the pedestrian crossing delay on a crosswalk during the peak hour as determined by the pedestrian level of service analysis in the HCM 2000.

 $[\]sim\!$ Volume exceeds capacity; queue is theoretically infinite.

^{*}SimTraffic results presented instead of Synchro results



Table E-1 presents the indicators for this criterion. Tables E-2 present the evaluation of PLOS criteria for each crosswalk at study area intersections under existing and full build conditions.

TABLE E-1 CRITERION – PLOS INDICATORS

Existing	With Project
PLOS A	PLOS A
PLOS B	PLOS B
PLOS C	PLOS C
PLOS D	PLOS D or increase of 3 seconds
PLOS E, F	PLOS D

TABLE E-2 STUDY AREA INTERSECTIONS PLOS SUMMARY

		Morning Peak Hour		Evening Peak Hour				
Intersection	Crosswalk	Existing	Build	Exceeds Criteria?	Existing	Build	Exceeds Criteria?	
	East	D	D	No	E	E	Yes	
Cambridgepark Drive/Steel	West	D	D	No	E	Е	Yes	
Place	North	D	D	No	Е	E	Yes	
	South	D	D	No	E	Е	Yes	
Cambridgepark Drive/Alewife Brook Parkway		No pedestrian facilities provided						
Alewife Brook	East	Е	Е	Yes	Е	Е	Yes	
Parkway/Rindge Avenue	South	E	Е	Yes	E	Е	Yes	
Alewife Brook Parkway at Route 2/16	East	E	E	Yes	E	E	Yes	
Cambridgepark Drive/100 Cambridgepark Drive Driveway	South	Α	В	Yes	В	В	No	
Cambridgepark Drive/Site West (outbound) Driveway	South	А	-	_	А	-	-	
Cambridgepark Drive/Site	West	F	-	-	Е	-	-	
East (inbound) Driveway	South	Α	-	-	Α	-	-	
Steel Place/Alewife Station	East	В	В	No	E	Е	Yes	
Access Road (Route 2	West	Α	Α	No	Α	Α	No	
Connector)	North	F	F	Yes	Е	Е	Yes	



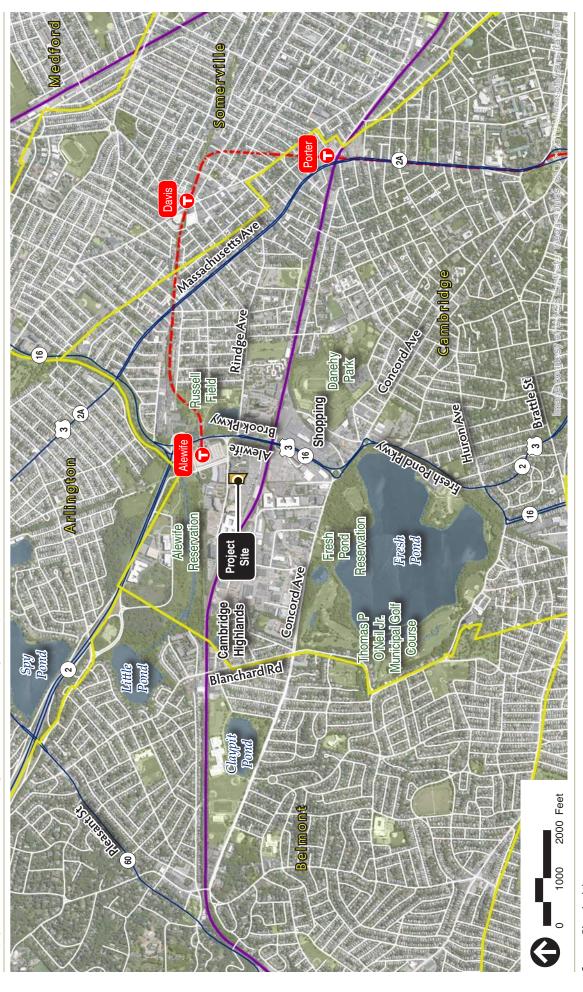
Criteria 2 & 3: Safe Pedestrian and Bicycle Facilities

Safe pedestrian and bicycle facilities are off-road or non-street bicycle lanes and sidewalks that are along a publicly-accessible street.

Table E-3 presents the indicators for this criterion. The evaluation of sidewalks or walkways and bicycle facilities are displayed.

TABLE E-3 PEDESTRIAN AND BICYCLE FACILITIES

Adjacent Street Link (between)		Sidewalk or	Exceeds	Bicycle Facilities or	Exceeds	
		Walkway Present	Criteria?	Right of Ways Present	Criteria?	
Cambridgepark Drive	Site Driveway	Yes	No	Yes	No	



Source: Bing Aerial

----- Key Regional Roadways

- MBTA Red Line

MBTA Commuter Rail

Whb Figure A

rigure A

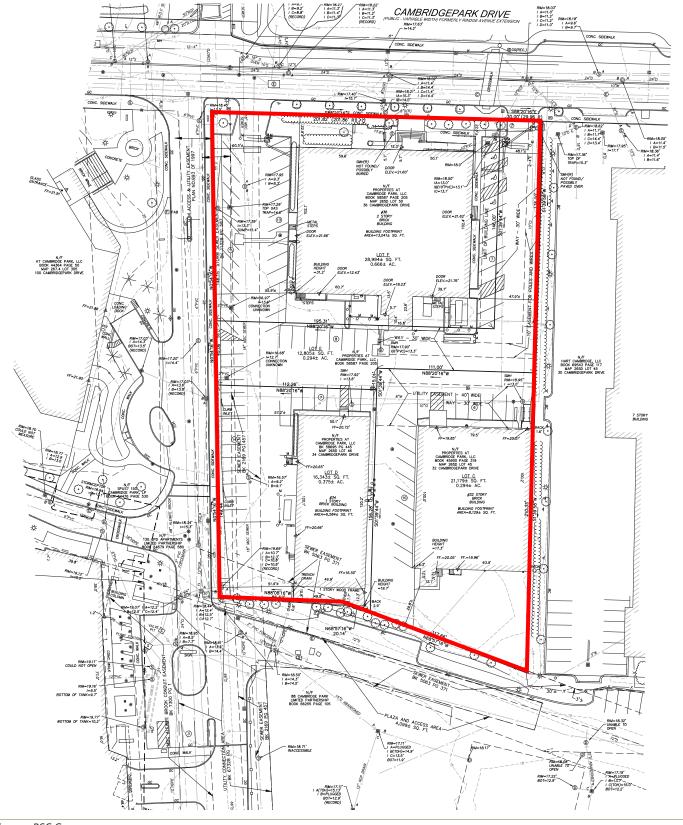
Site Location Map

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Project Site

Source: World Aerial



Source: BSC Group



Figure C

Existing Conditions Site Plan



Source: BSC Group

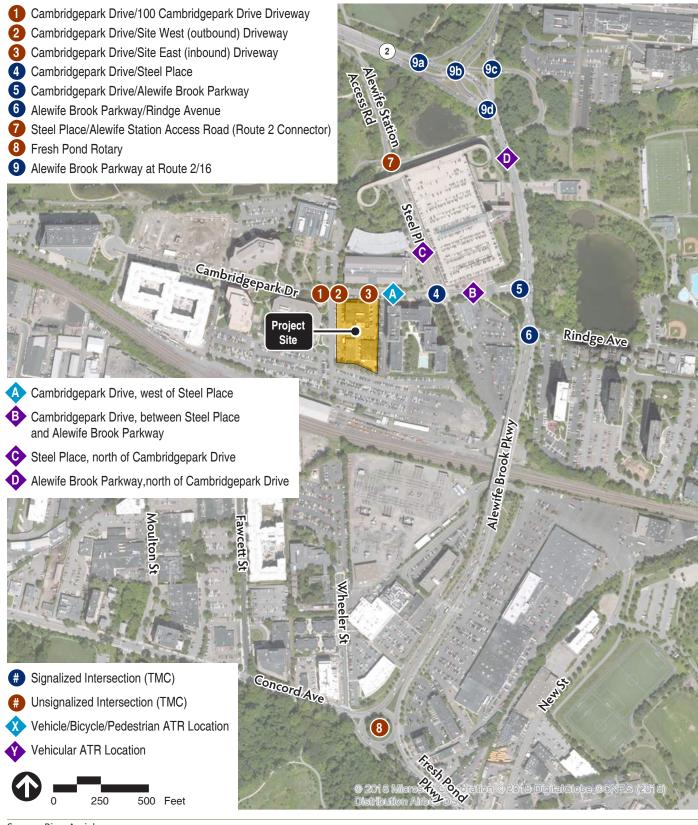


▲ Vehicle Access



Figure D

Proposed Site Plan



Source: Bing Aerial

Turning Movement Conut = TMC Automatic Traffic Recorder = ATR



Figure E TIS Study Area Intersections

Cambridgepark Dr



Shared Driveway

Source: Cube 3 Studio



Figure F.1

Proposed Vehicular Parking Plans Ground Floor

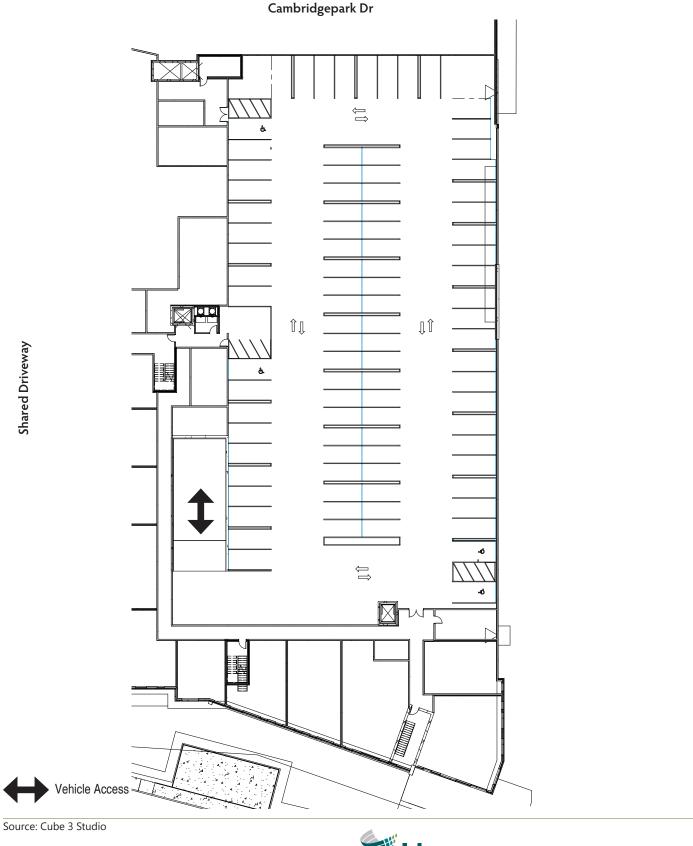
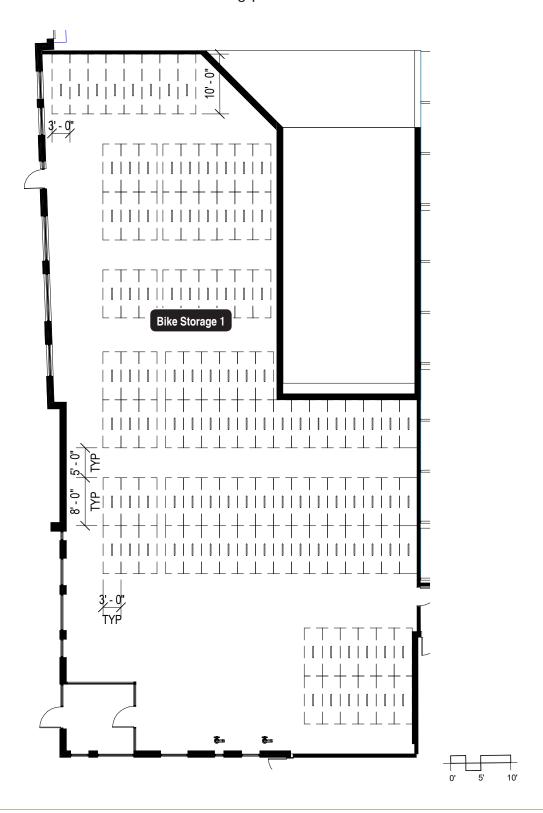




Figure F.2

Proposed Vehicualr Parking Plans Second Floor

Cambridgepark Dr

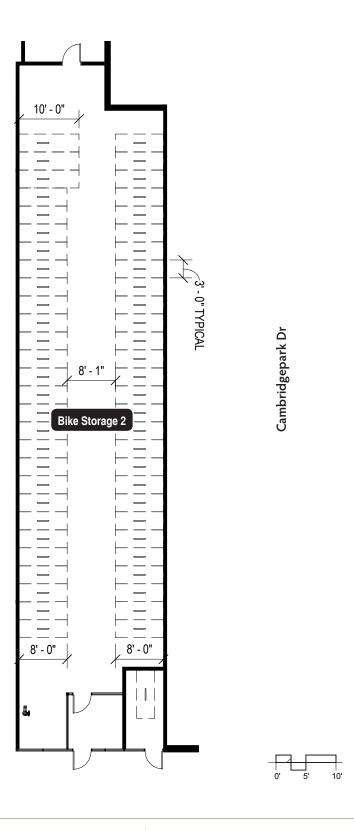


Source: Cube 3 Studio



Figure G.1

Proposed Long-Term Bike Parking



Source: Cube 3 Studio



Figure G.2
Proposed Long-Term Bike Parking





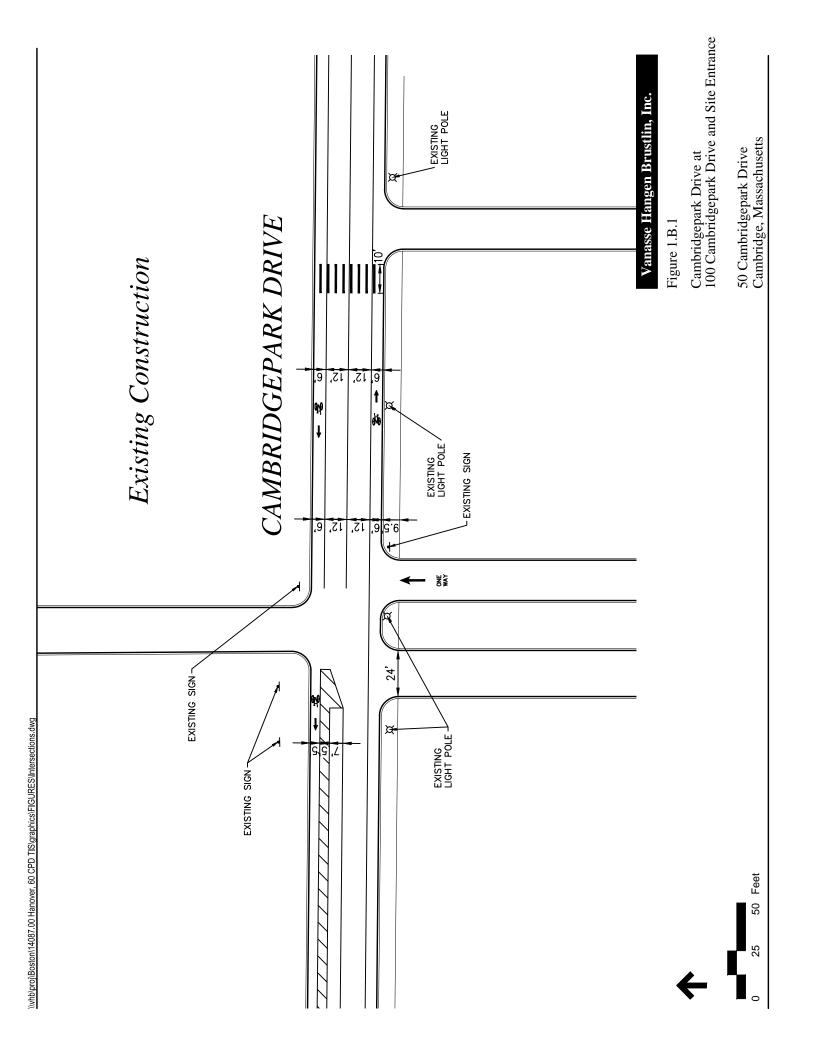
Short-Term Bicycle Spaces

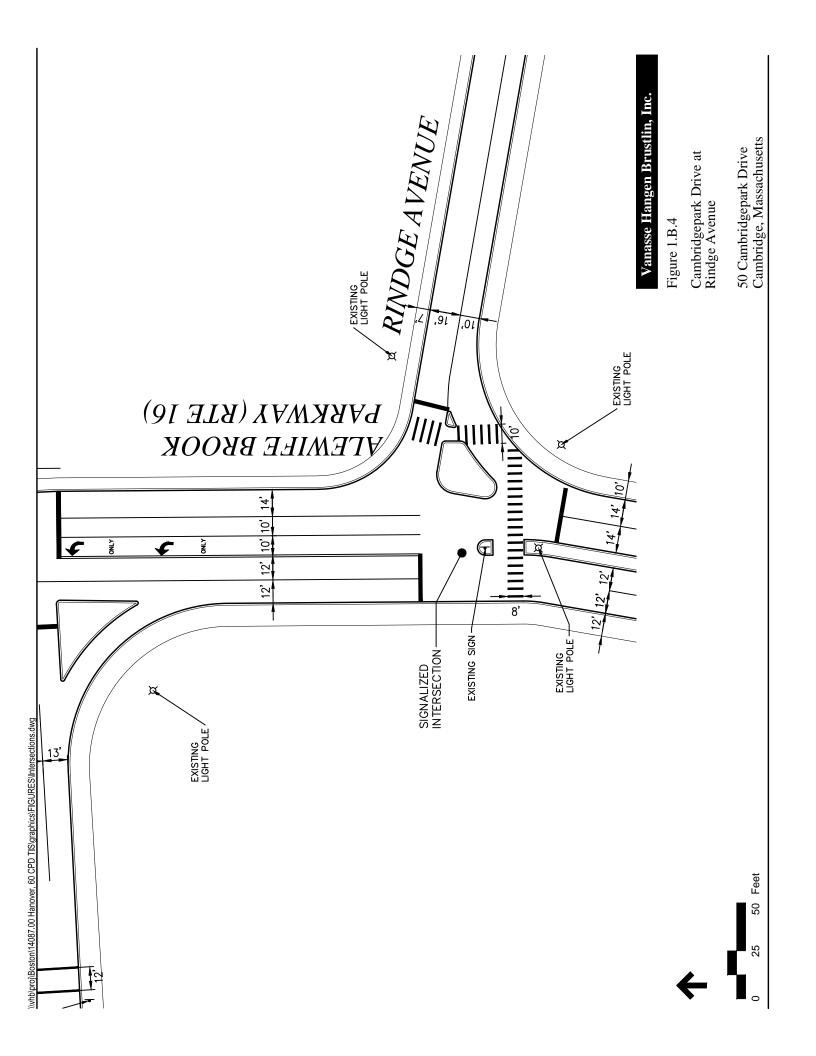
Source: Cube 3 Studio

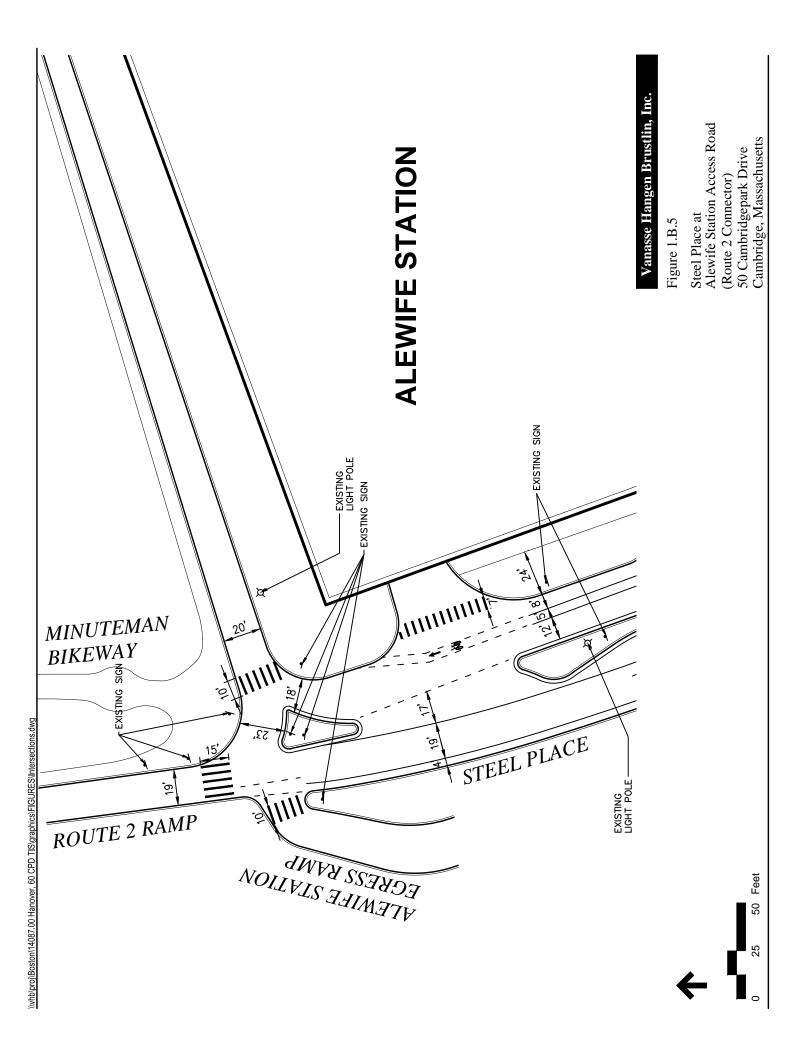


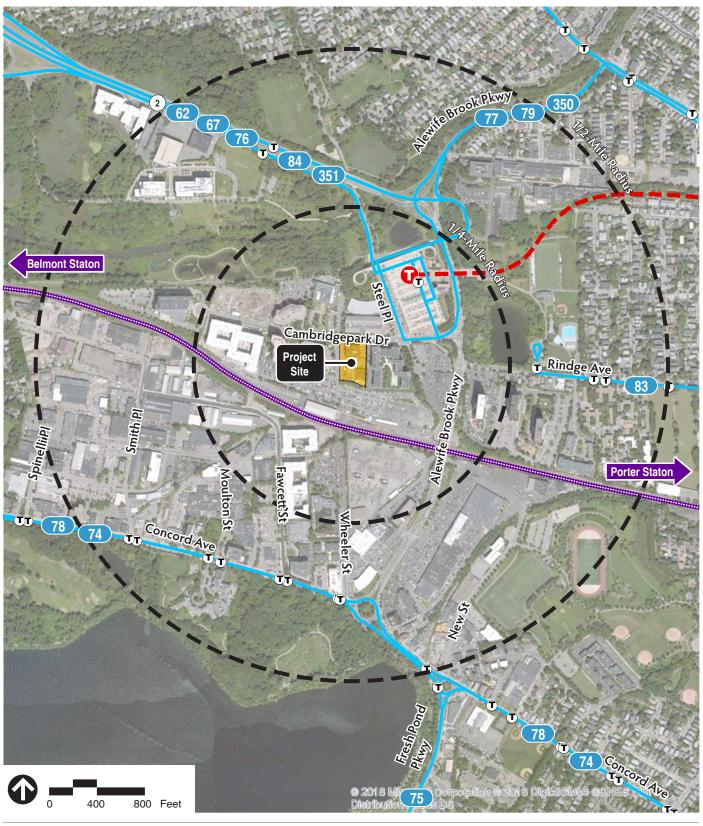
Figure G.3

Proposed Short-Term Bike Parking



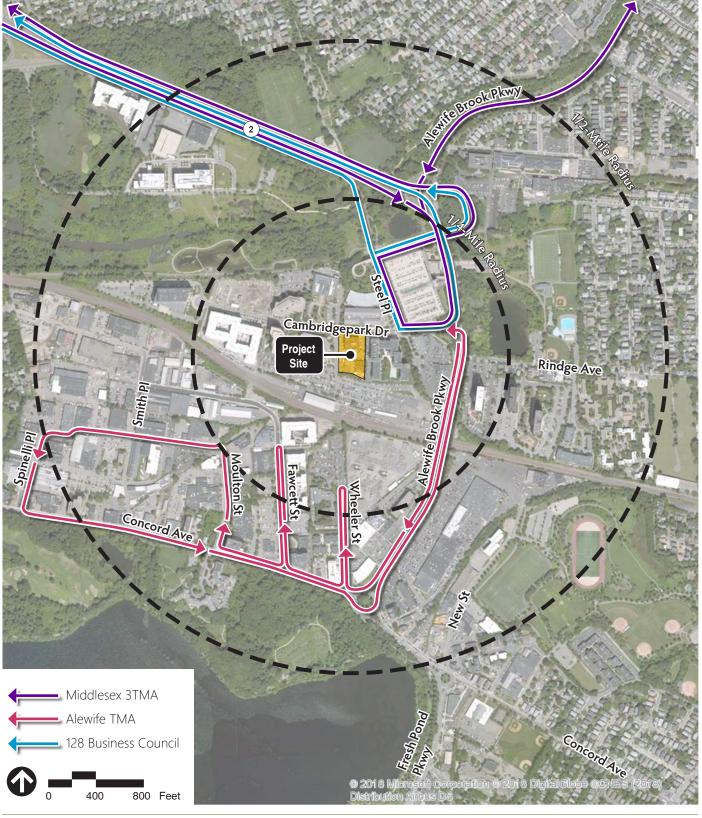






Source: Bing Aerial, MBTA





Source: Bing Aerial, Alewifetma.org



Figure 1.d.2 Private Transit Services



Source: Bing Aerial, Hubway.com, Zipcar.com



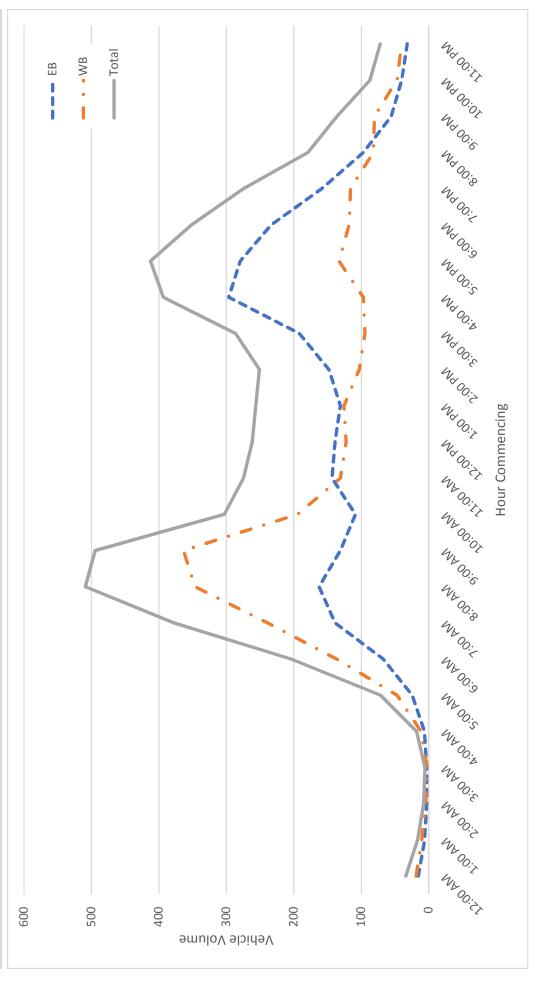
Figure 1.d.3

Bike and Car Sharing Services



Source: Bing Aerial 2014, City of Cambridge GIS

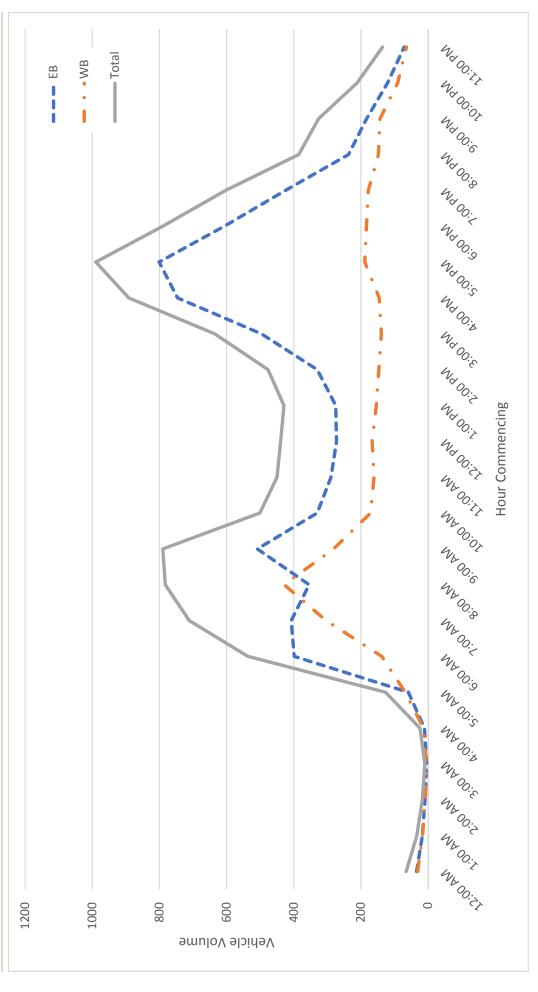




Note: Vehicle volumes are the average volume from the 48-hour ATR collected on Nov 15 and 16, 2017



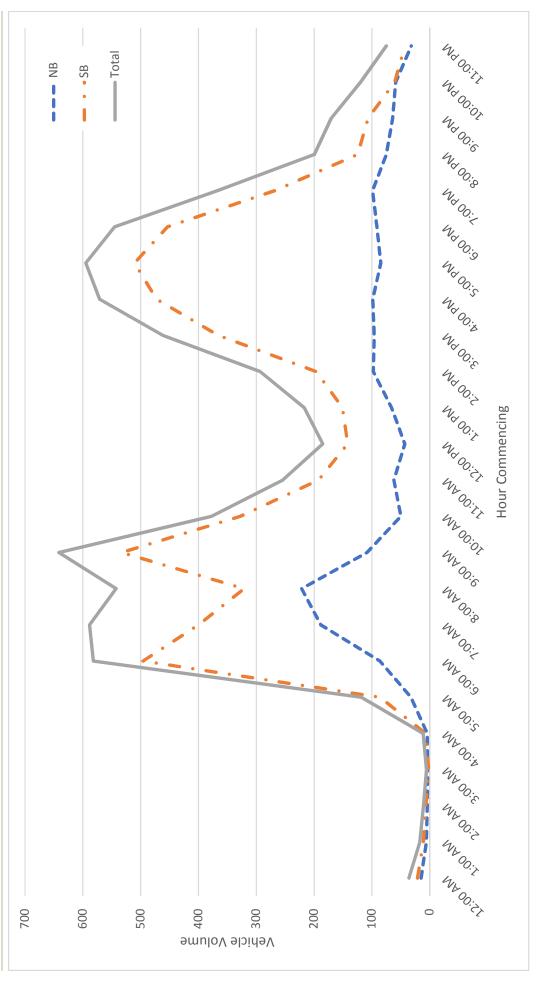
Cambridgepark Drive, West of Steel Place Daily ATR Summary



Note: Vehicle volumes are the average volume from the 48-hour ATR collected on Nov 15 and 16, 2017



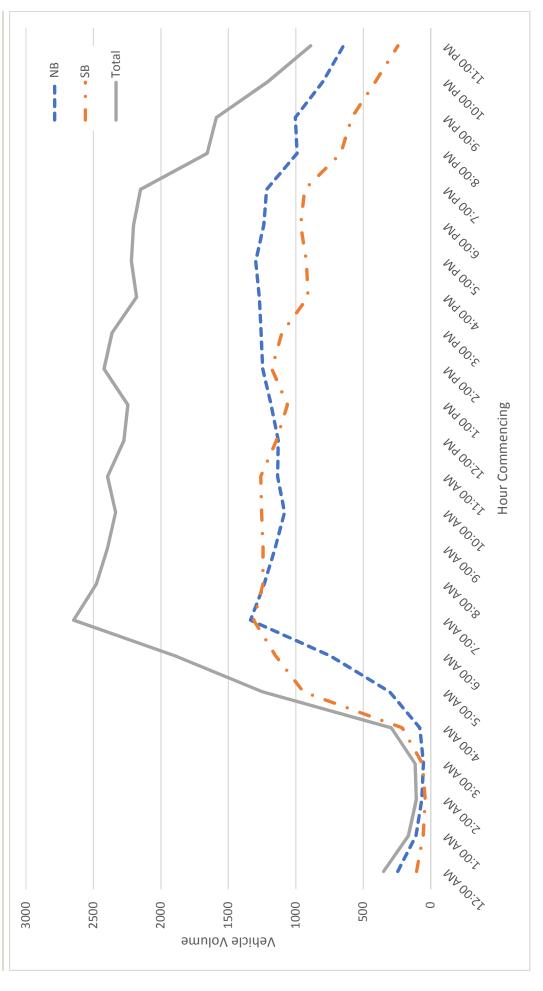
Cambridgepark Drive, East of Steel Place Daily ATR Summary



Note: Vehicle volumes are the average volume from the 48-hour ATR collected on Nov 15 and 16, 2017



Steel Place, North of Cambridgepark Drive Daily ATR Summary



Note: Vehicle volumes are the average volume from the 48-hour ATR collected on Nov 15 and 16, 2017



Alewife Brook Parkway, North of Cambridgepark Drive-Daily ATR Summary

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Note: Vehicle volumes are the average volume from the 48-hour ATR collected on Nov 15 and 16, 2017



Figure 2.a.5

Cambridgepark Drive, West of Steel Place PM Peak Period Summary

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Note: Vehicle volumes are the average volume from the 48-hour ATR collected on Nov 15 and 16, 2017



Figure 2.a.6

Cambridgepark Drive, East of Steel Place PM Peak Period Summary

\\vhb\proj\Boston\14087.00 Hanover, 60 CPD TIS\graphics\FIGURES\TIS\Traffic figures.indd p21 04/06/18

Note: Vehicle volumes are the average volume from the 48-hour ATR collected on Nov 15 and 16, 2017



Figure 2.a.7

Steel Place, North of Cambridgepark Drive PM Peak Period Summary

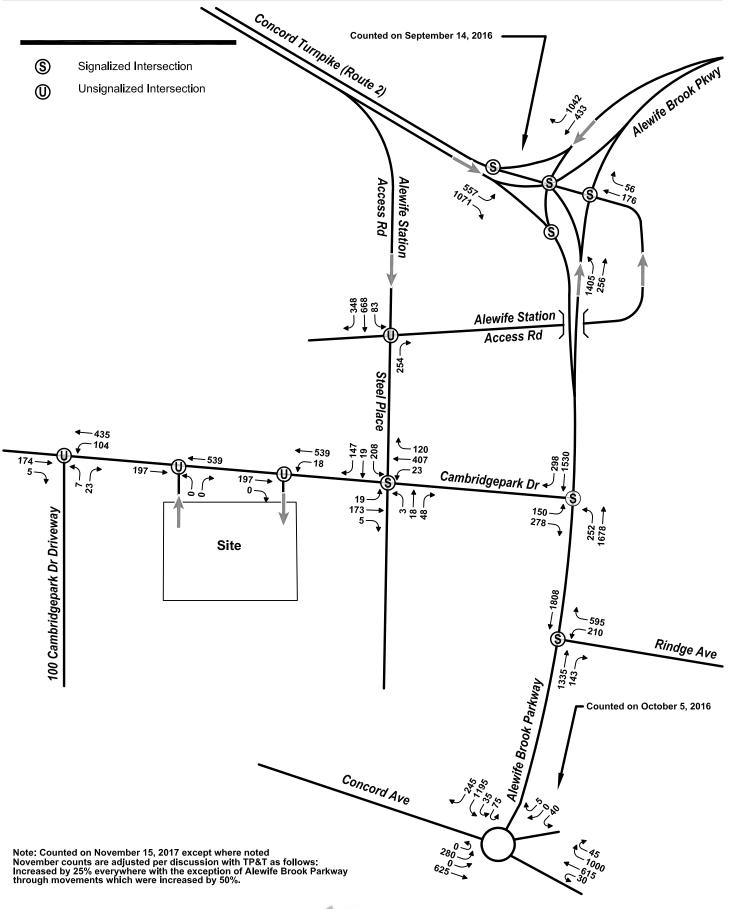
\\vhb\proj\Boston\14087.00 Hanover, 60 CPD TIS\graphics\FIGURES\TIS\Traffic figures.indd p22 04/06/18

Note: Vehicle volumes are the average volume from the 48-hour ATR collected on Nov 15 and 16, 2017



Figure 2.a.8

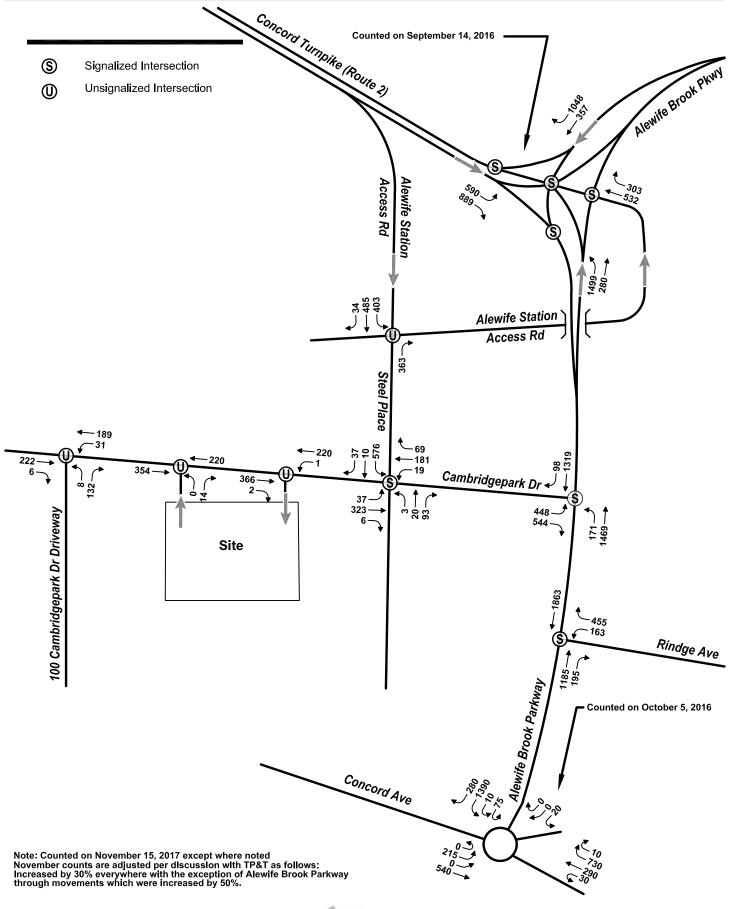
Alewife Brook Parkway, North of Cambridgepark Drive PM Peak Period Summary







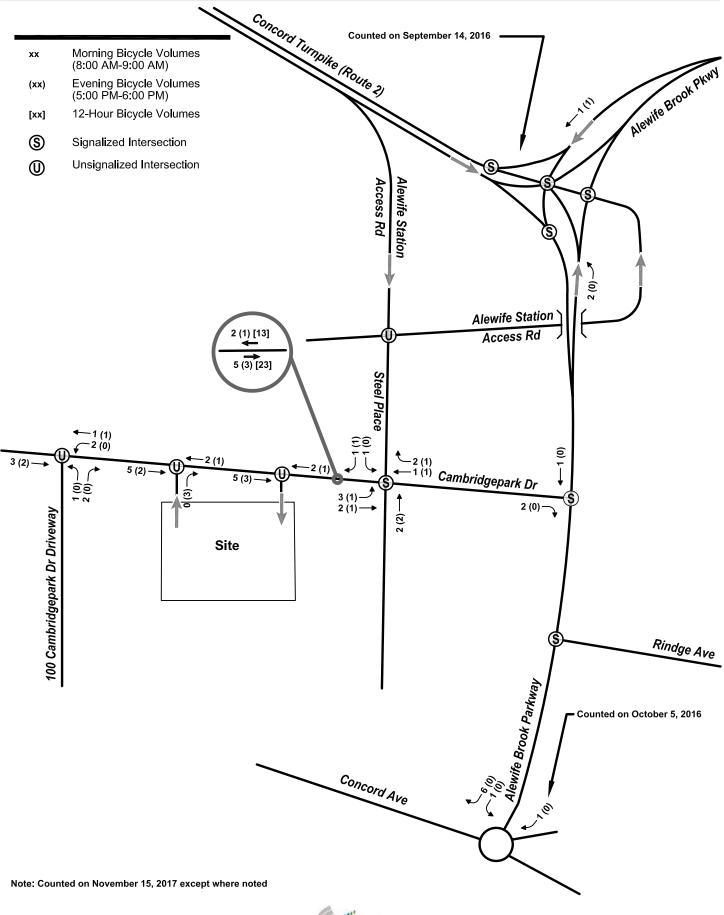
2018 Existing Condition Morning Peak Hour Vehicle Volumes



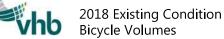




2018 Existing Condition Evening Peak Hour Vehicle Volumes







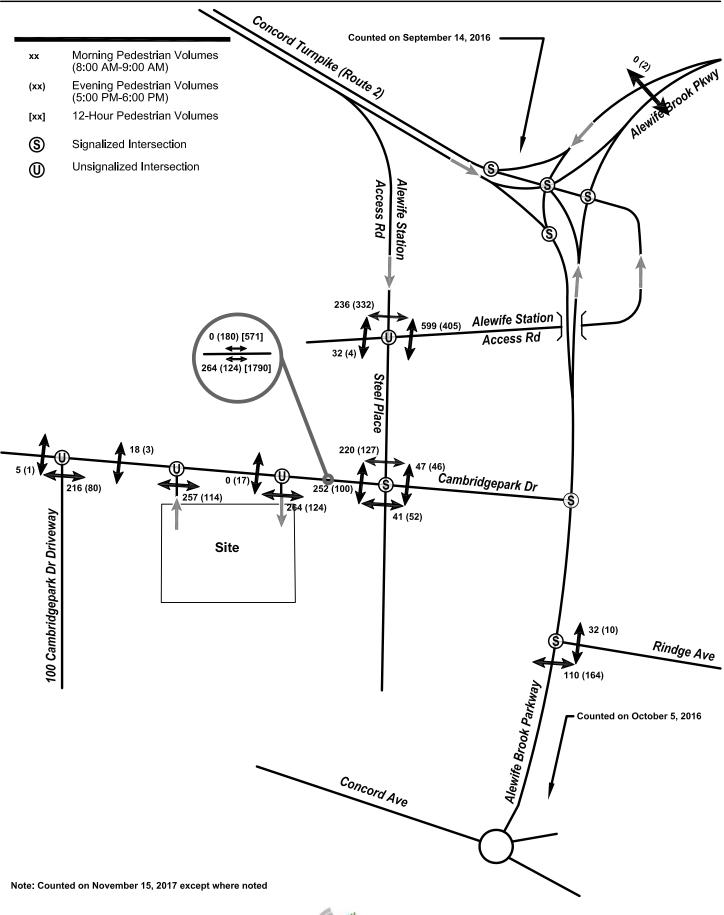








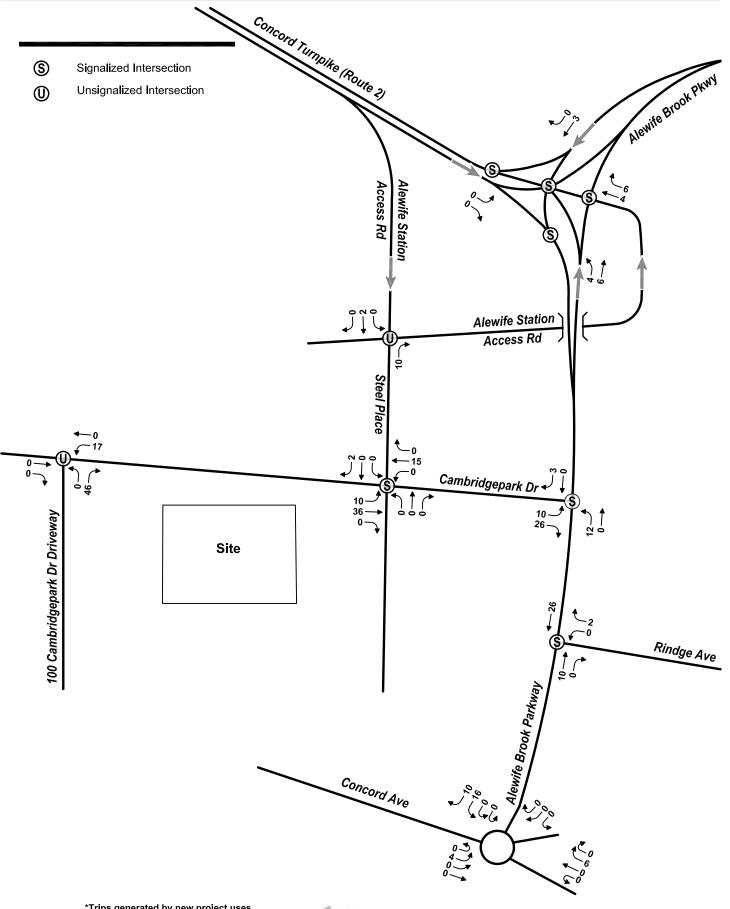


Figure 3.c.1
Project Trip Distribution





Figure 3.c.2 Existing Site Trip Distribution

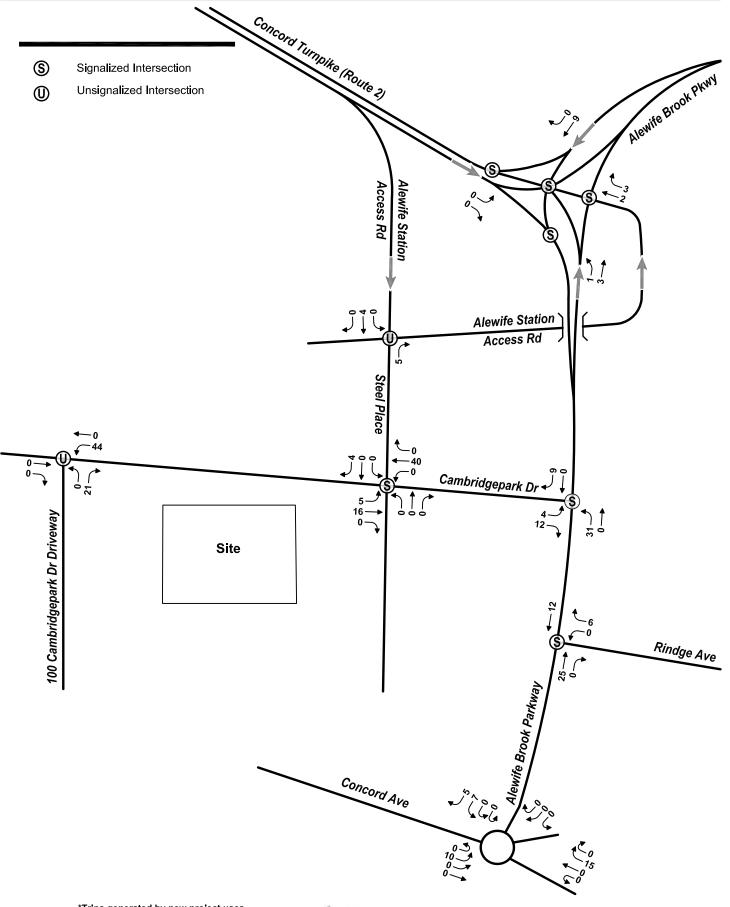


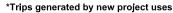






Total Project Generated Trips* Morning Peak Hour Vehicle Volumes

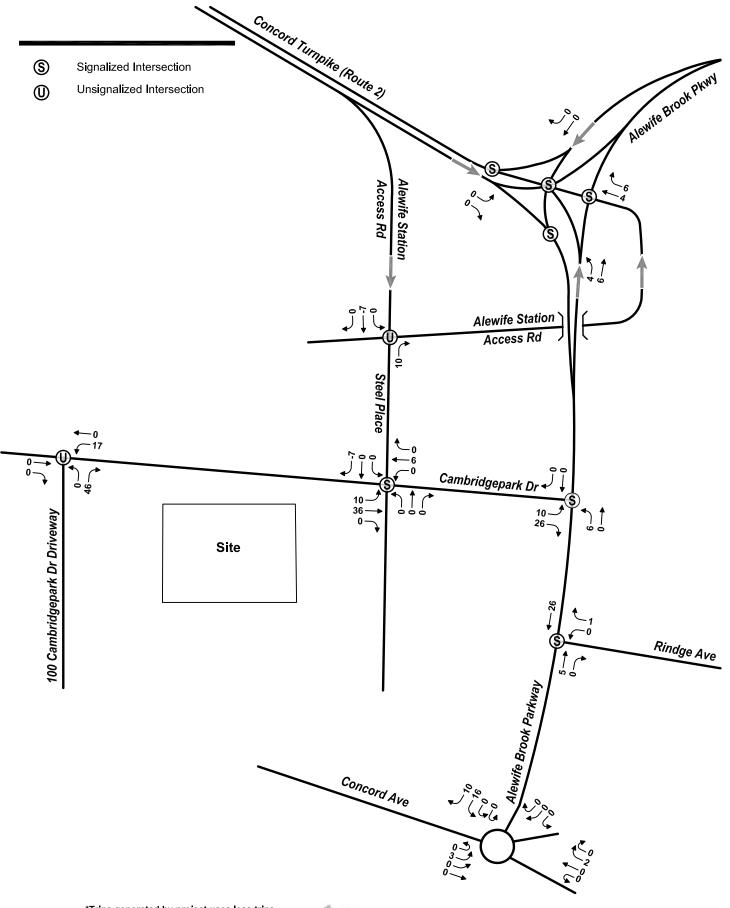


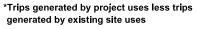






Total Project Generated Trips* Evening Peak Hour Vehicle Volumes

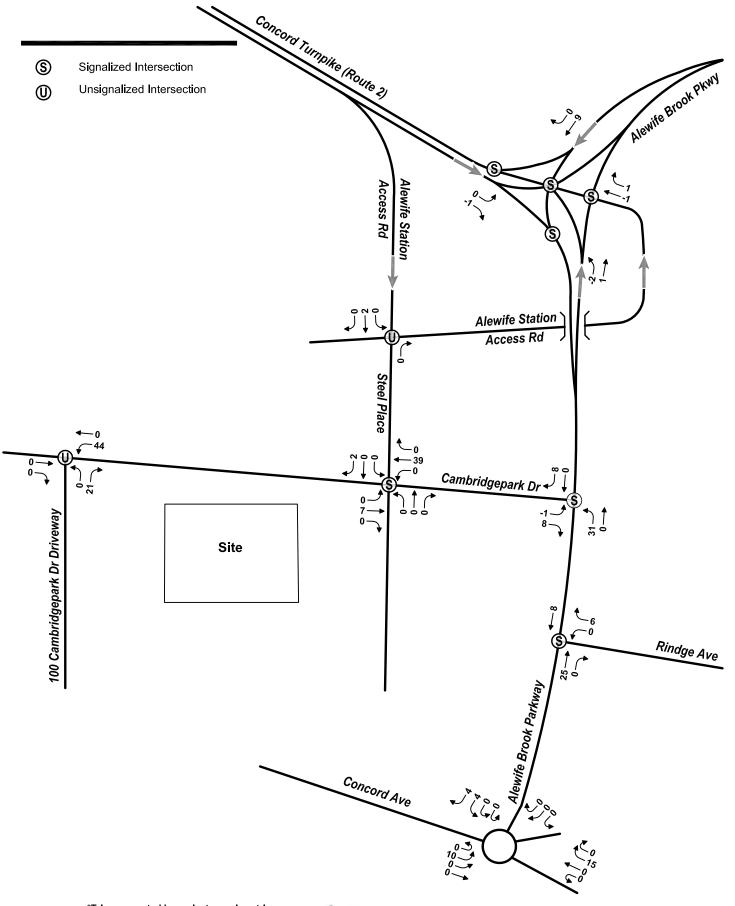


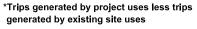


Not to Scale



Net New Project Generated Trips* Morning Peak Hour Vehicle Volumes





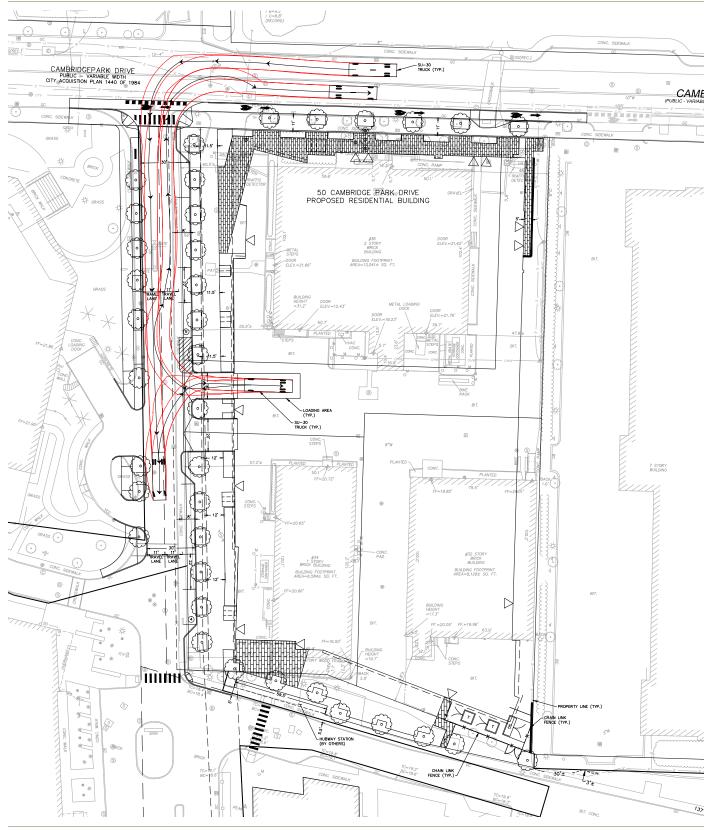
Not to Scale



Net New Project Generated Trips* Evening Peak Hour Vehicle Volumes



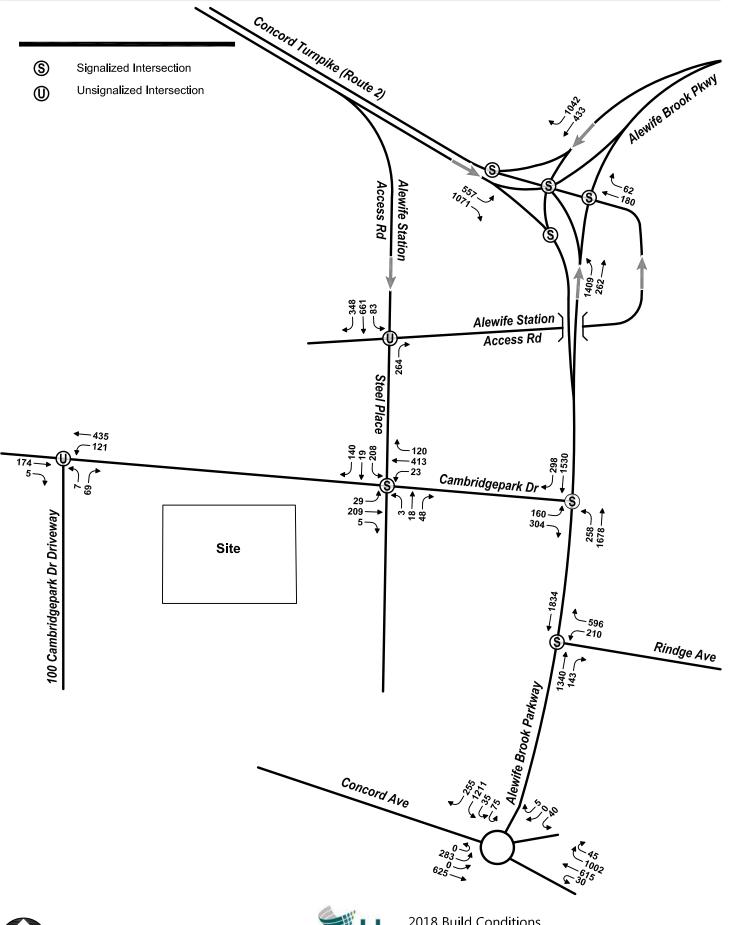
Figure 3.d.1 Service and Loading



Source: Cube 3 Studio

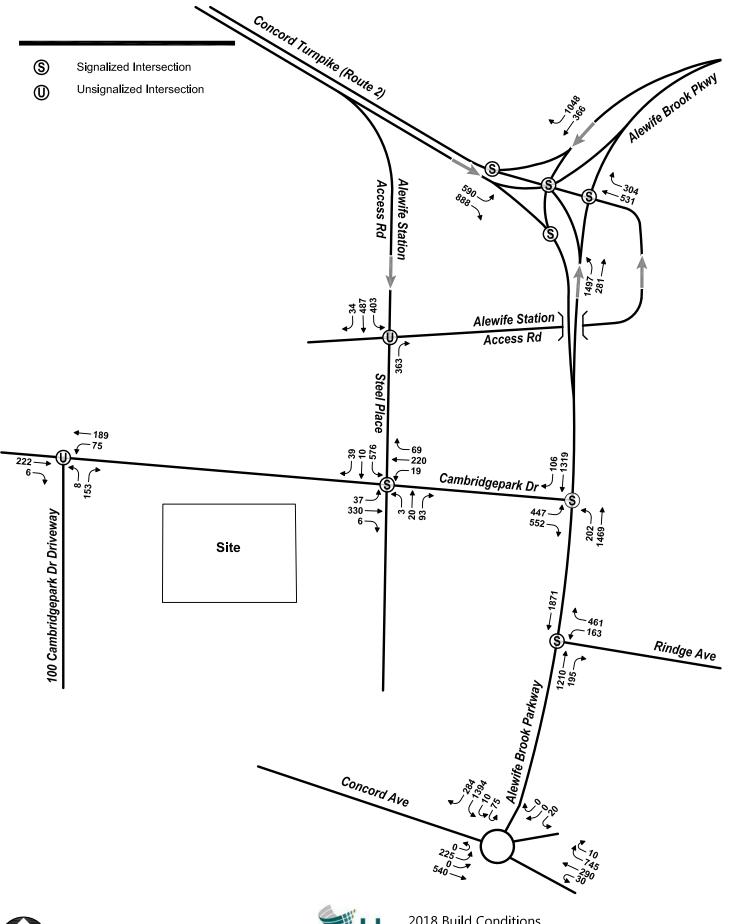


Figure 3.d.2 Loading Dock Truck Turns



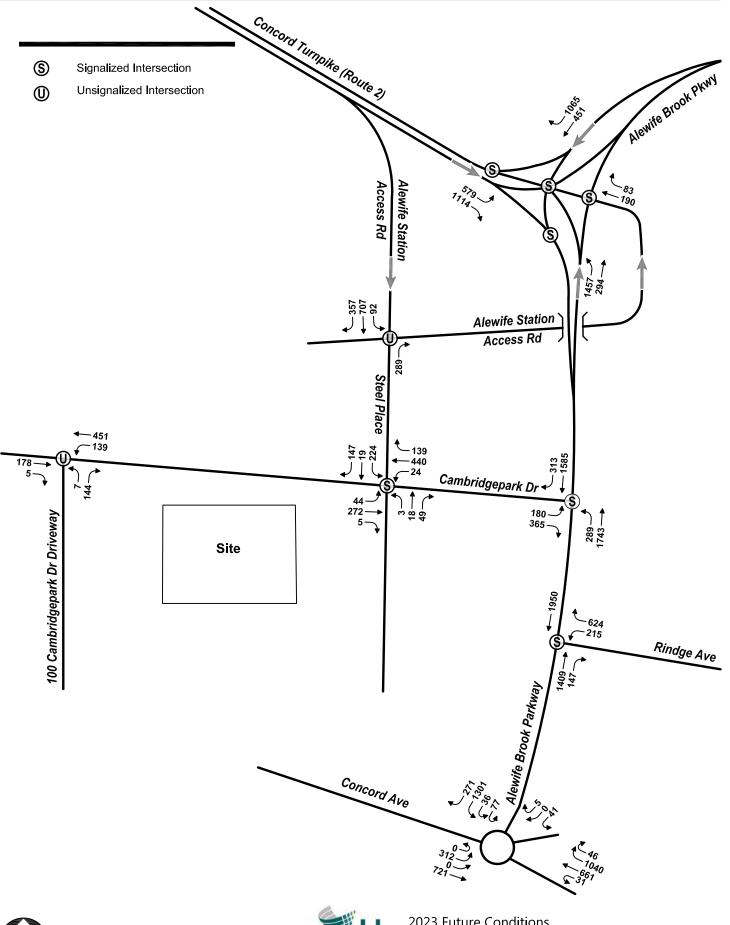


2018 Build Conditions Morning Peak Hour Vehicle Volumes



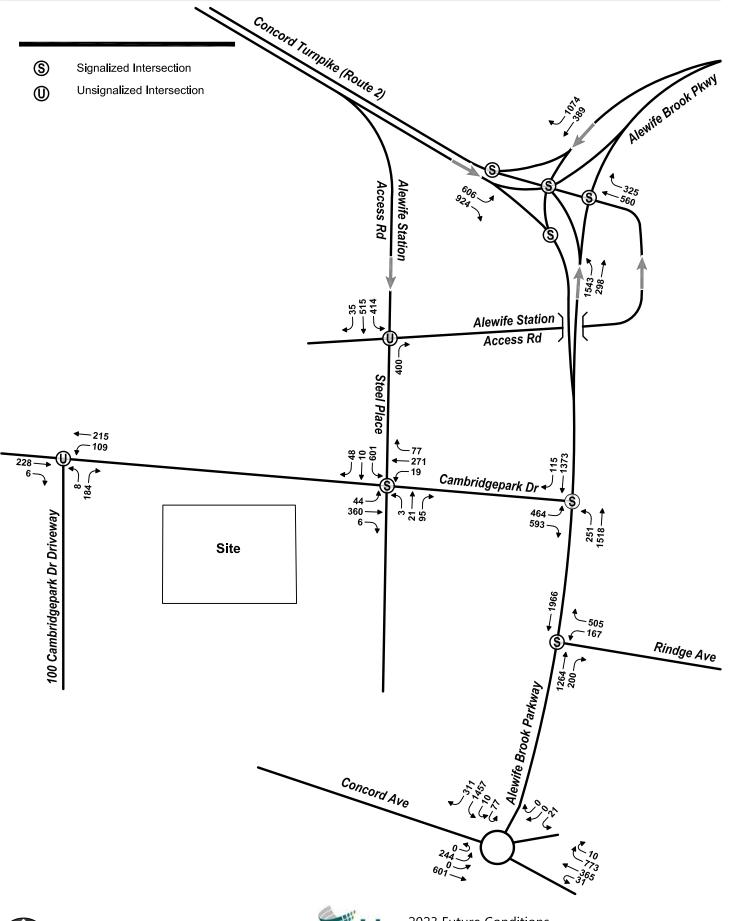


2018 Build Conditions Evening Peak Hour Vehicle Volumes





2023 Future Conditions Morning Peak Hour Vehicle Volumes





2023 Future Conditions Evening Peak Hour Vehicle Volumes





Figure 5.c.3

Cumulative Area Developments Impact Evening Peak Hour Vehicle Volumes

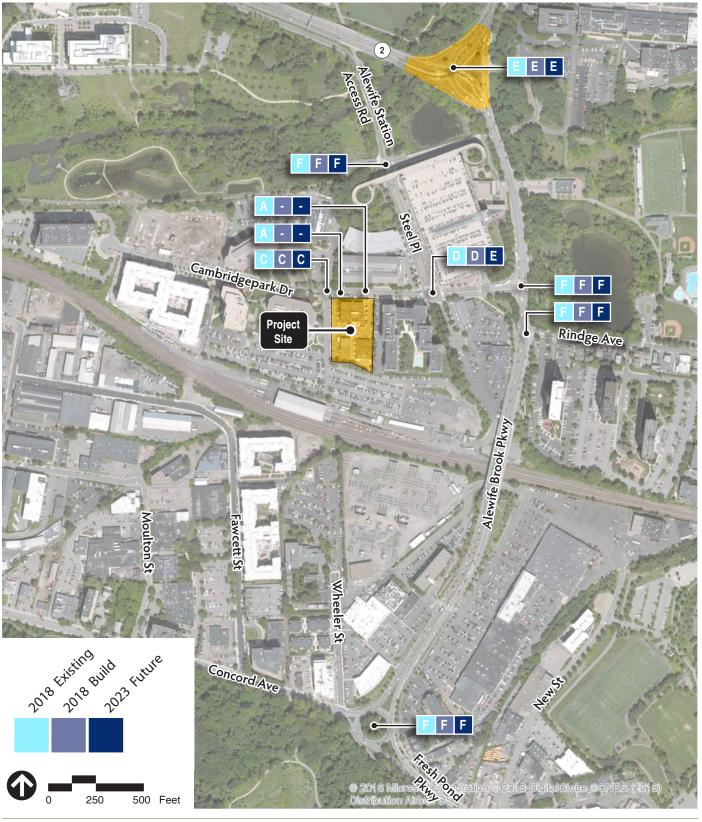




Figure 6.a.1

AM Peak Vehicle Level of Service

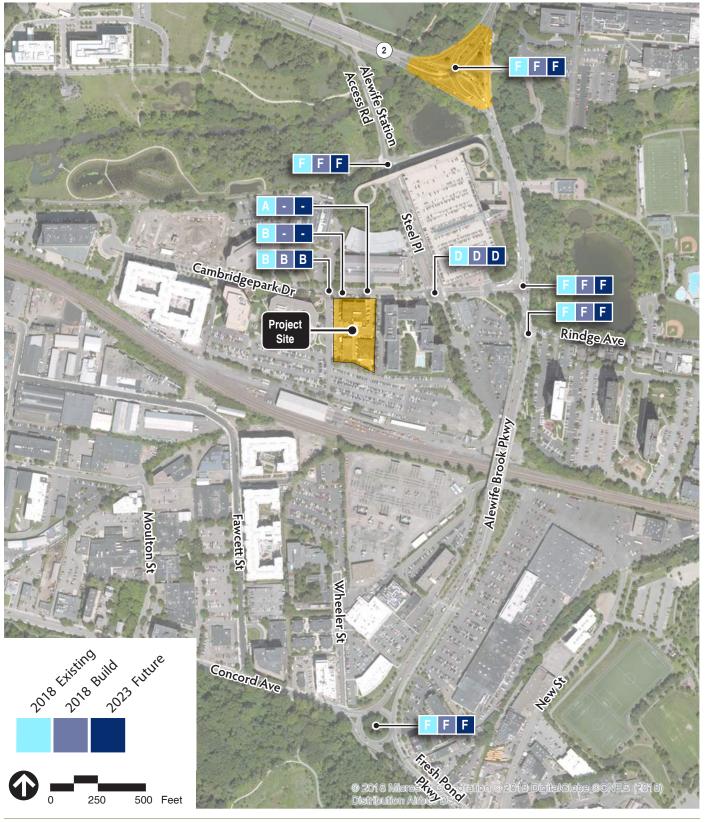




Figure 6.a.2

PM Peak Vehicle Level of Service

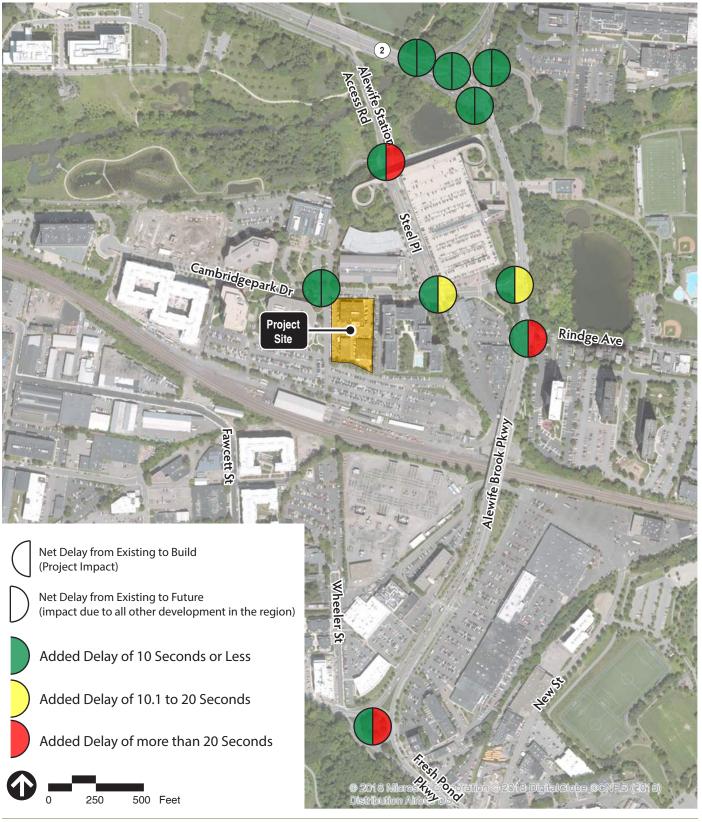




Figure 6.b.1

AM Peak Net Change in Vehicular Delay

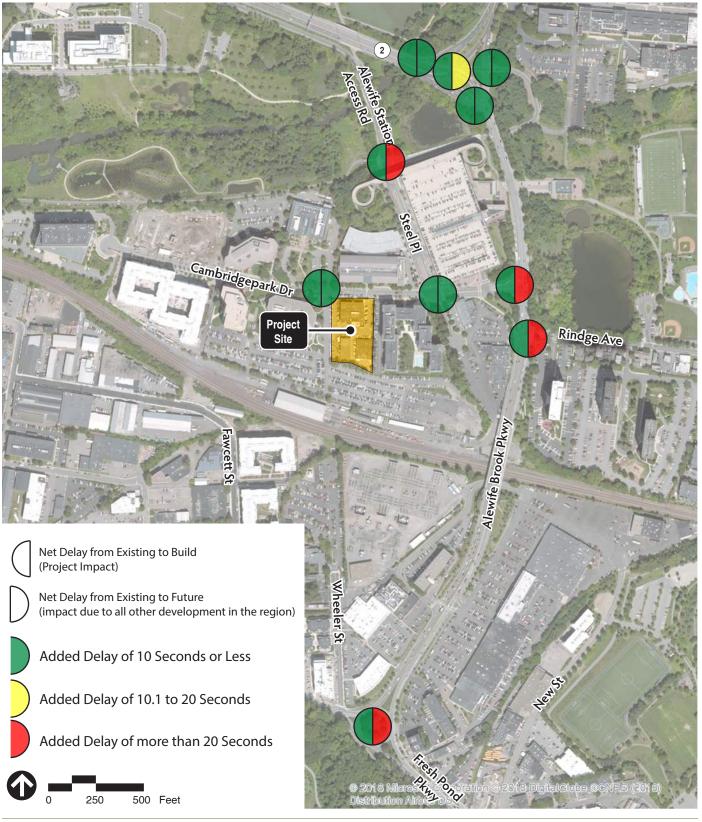




Figure 6.b.2 PM Peak Net Change in Vehicular Delay

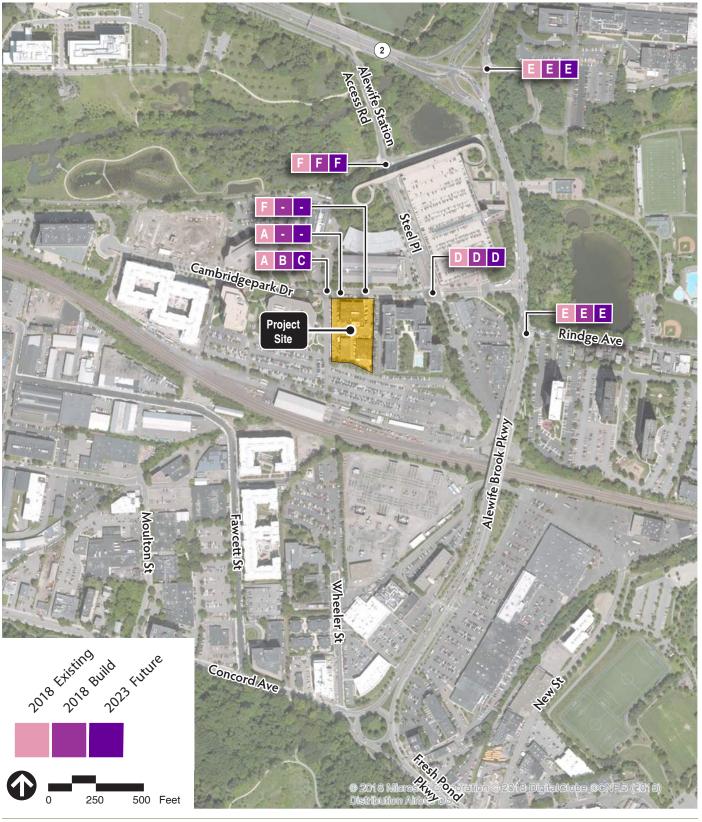




Figure 11.a.1

AM Peak Pedestrian Level of Service

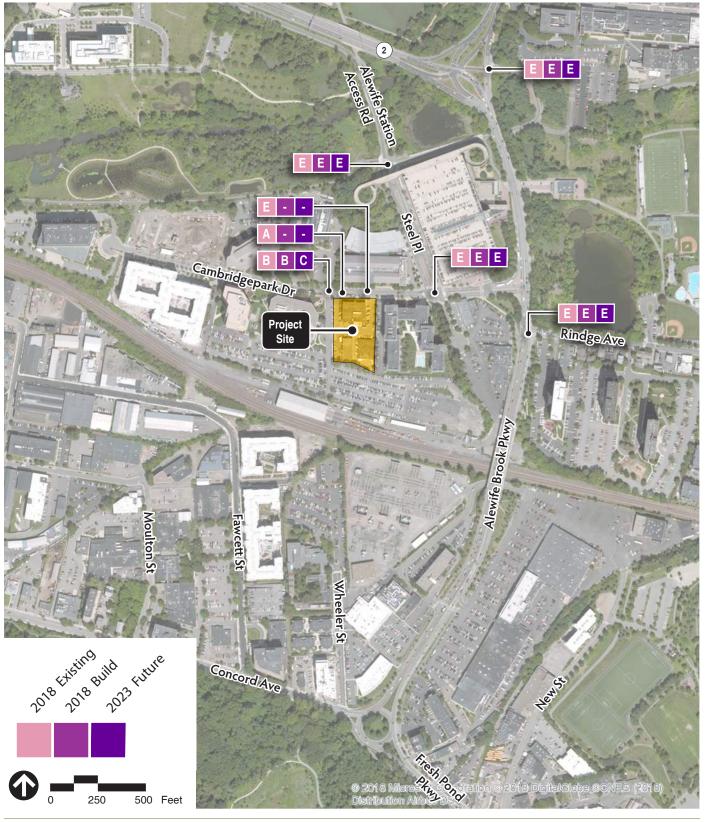
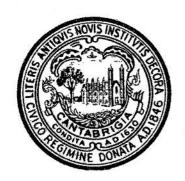




Figure 11.a.2

PM Peak Pedestrian Level of Service

Appendix C Certification of Receipt of Plans



PLANNING BOARD

CITY HALL ANNEX, 344 BROADWAY, CAMBRIDGE, MA 02139

CERTIFICATION OF RECEIPT OF PLANS BY CITY OF CAMBRIDGE TRAFFIC, PARKING & TRANSPORTATION

City Department/Office: Traffic, Parking & Transportation

Project Address: 50 CambridgePark Drive

Applicant Name: The McKinnon Company on behalf of The Hanover Company

For the purpose of fulfilling the requirements of Section 19.20 and/or 6.35.1 and/or 5.28.2 of the Cambridge Zoning Ordinance, this is to certify that this Department is in receipt of the application documents submitted to the Planning Board for approval of a Project Review Special Permit for the above referenced development project: (a) an application narrative, (b) small format application plans at 11" x 17" or the equivalent and (c) Certified Traffic Study. The Department understands that the receipt of these documents does not obligate it to take any action related thereto.

Signature of City Department/Office Representative



PLANNING BOARD

CITY HALL ANNEX, 344 BROADWAY, CAMBRIDGE, MA 02139

CERTIFICATION OF RECEIPT OF PLANS BY CITY OF CAMBRIDGE DEPARTMENT OF PUBLIC WORKS

City Department/Office: Department of Public Works

Project Address: 50 CambridgePark Drive

Applicant Name: The McKinnon Company on behalf of The Hanover Company

For the purpose of fulfilling the requirements of Section 19.20 of the Cambridge Zoning Ordinance, this is to certify that this Department is in receipt of the application documents submitted to the Planning Board for approval of a Project Review Special Permit for the above referenced development project: (a) an application narrative and (b) small format application plans at 11" x 17" or the equivalent. The Department understands that the receipt of these documents does not obligate it to take any action related thereto.

Signature of City Department/Office Representative



PLANNING BOARD

CITY HALL ANNEX, 344 BROADWAY, CAMBRIDGE, MA 02139

CERTIFICATION OF RECEIPT OF PLANS BY CITY OF CAMBRIDGE TREE ARBORIST

City Department/Office: City of Cambridge Tree Arborist

Project Address: 50 CambridgePark Drive

Applicant Name: The McKinnon Company on behalf of The Hanover Company

For the purpose of fulfilling the requirements of Section 4.26, 19.20 or 11.10 of the Cambridge Zoning Ordinance, this is to certify that this Department is in receipt of the application documents submitted to the Planning Board for approval of a MultiFamily, Project Review or Townhouse Special Permit for the above referenced development project: a Tree Study which shall include (a) Tree Survey, (b) Tree Protection Plan and if applicable, (c) Mitigation Plan, twenty one days before the Special Permit application to Community Development.

Signature of City Department/Office Representative



PLANNING BOARD

CITY HALL ANNEX, 344 BROADWAY, CAMBRIDGE, MA 02139

CERTIFICATION OF RECEIPT OF PLANS BY CITY OF CAMBRIDGE WATER DEPARTMENT

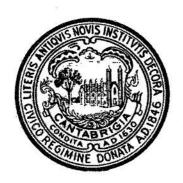
City Department/Office: Water Department

Project Address: 50 CambridgePark Drive

Applicant Name: The McKinnon Company on behalf of The Hanover Company

For the purpose of fulfilling the requirements of Section 19.20 of the Cambridge Zoning Ordinance, this is to certify that this Department is in receipt of the application documents submitted to the Planning Board for approval of a Project Review Special Permit for the above referenced development project: (a) an application narrative and (b) small format application plans at 11" x 17" or the equivalent. The Department understands that the receipt of these documents does not obligate it to take any action related thereto.

Signature of City Department/Office Representative



PLANNING BOARD

CITY HALL ANNEX, 344 BROADWAY, CAMBRIDGE, MA 02139

CERTIFICATION OF RECEIPT OF PLANS BY CITY OF CAMBRIDGE LEED SPECIALIST

City Department/Office: Community Development Department

Project Address: 50 CambridgePark Drive

Applicant Name: The McKinnon Company on behalf of The Hanover Company

For the purpose of fulfilling the requirements of Section 22.20 of the Cambridge Zoning Ordinance, this is to certify that this Department is in receipt of the application documents submitted to the Planning Board for approval of a Special Permit for the above referenced development project: (a) an application narrative, (b) small format application plans at 11" x 17" or the equivalent and (c) completed LEED Project Checklist for the appropriate LEED building standard, accompanying narrative and affidavit. The Department understands that the receipt of these documents does not obligate it to take any action related thereto.

Signature of City Department/Office Representative