GENERAL CONTRACTOR

Thoughtforms Corporation 525 Massachusetts Ave, Suite 204 Acton, MA 01720 978.263.6019

SURVEY/SITE ENGINEER

Hancock Associates 185 Centre Street Danvers, MA 01923 978.777.3050

ARCHITECT

Hart Associates Architects Inc. 50 Church Street Belmont, MA 02478 617.489.0030

STRUCTURAL ENGINEER

Siegel Associates 860 Walnut Street Newton Centre, MA 02459 617.244.1612

12 Lakeview Avenue

CAMBRIDGE, MA

REISSUED PERMIT SET

June 16, 2021 AMENDED 03.17.2022 WITH REDUCED NORTH COVERED PORCH AND ADDED SOUTH COVERED PORCH

LIST OF DRAWINGS:

TITLE SHEET

SITE DRAWING

- EC EXISTING CONDITIONS PLAN OF LAND IN CAMBRIDGE, MA
- PROPOSED PLOT PLAN OF LAND IN CAMBRIDGE, MA
- DRAINAGE AND UTILITY PLAN **C**1 **C**2 DRAINAGE AND UTILITY DETAILS

ARCHITECTURE DRAWINGS

- EX1.0 EXISTING BASEMENT PLAN
- **EXISTING FIRST FLOOR PLAN**
- EX1.2 EXISTING SECOND FLOOR PLAN
- **EXISTING THIRD FLOOR PLAN** EX1.3 EX1.4 EXISTING CARRIAGE HOUSE PLAN AND ELEVATIONS
- EX2.1 **EXISTING SOUTH & EAST ELEVATIONS EXISTING NORTH & WEST ELEVATIONS EX2.2**
- D1.0 BASEMENT DEMOLITION PLAN
- D1.1 FIRST FLOOR DEMOLITION PLAN
- SECOND FLOOR DEMOLITION PLAN D1.2 THIRD FLOOR DEMOLITION PLAN D1.3
- D2.1 **DEMOLITION SOUTH & EAST ELEVATIONS** D2.2 **DEMOLITION NORTH & WEST ELEVATIONS**
- FOUNDATION PLAN (Updated 05.28.2021) A0.1 FOUNDATION DETAILS (Updated 05.28.2021) A0.2
- PROPOSED BASEMENT PLAN (Updated 05.28.2021) A1.0
- PROPOSED FIRST FLOOR PLAN UPDATED 03.17.2022 A1.1
- A1.2 PROPOSED SECOND FLOOR PLAN
- A1.3 PROPOSED THIRD FLOOR PLAN
- PROPOSED ROOF PLAN UPDATED 03.17.2022 A1.4
- PROPOSED EAST ELEVATION A2.1
- A2.2 PROPOSED SOUTH ELEVATION UPDATED 03.17.2022
- PROPOSED NORTH ELEVATION UPDATED 03.17.2022 A2.3
- PROPOSED WEST ELEVATION (Updated 05.28.2021) A2.4 A2.5 WINDOW AND DOORS SCHEDULE
- A3.1 **BUILDING SECTIONS**
- A3.2 **BUILDING SECTIONS**
- **BUILDING SECTIONS** A3.3
- A5.1 LIFE SAFETY PLANS (Added 03.26.2021)

STRUCTURAL DRAWINGS

- S0.01 GENERAL NOTES
- S1.01 FIRST FLOOR FRAMING PLAN
- S1.02 SECOND FLOOR FRAMING PLAN S1.03 THIRD FLOOR FRAMING PLAN
- S1.04 ROOF FRAMING PLAN
- S3.01 SECTIONS
- S3.02 SECTIONS

CALCULATION DRAWINGS

- PA-1 EXISTING & PROPOSED SQUARE FOOTAGE CALCULATIONS UPDATED 03.17.2022
- EXISTING & PROPOSED VOLUME CALCULATIONS (Added 05.28.2021) PA-2
- PA-3 EXISTING & PROPOSED HEIGHT CALCULATIONS (Added 05.28.2021)



ASSESSORS: MAP 234, LOT 80

REFERENCES: DEED BOOK 71824, PAGE 24 PLAN 1136 OF 1965

<u>RECORD</u> OWNER: JEFFERSON M & ELIZABETH GREEN CASE <u>ZONING:</u> RESIDENCE A-1

NOTES:

ELEVATION IS 49.40 (CCB).

1) ELEVATIONS SHOWN HEREON REFER TO THE CITY OF CAMBRIDGE BASE. PROJECT BENCH MARK IS AN "X" CUT IN OHW POLE ON NORTHWEST CORNER, ON THE NORTH SIDE OF STEP IN POLE, AT THE INTERSECTION OF LAKEVIEW AND BRATTLE.

2) UNDERGROUND UTILITIES SHOWN HEREON ARE COMPILED FROM FIELD LOCATIONS OF STRUCTURES AND FROM AVAILABLE RECORD INFORMATION ON FILE AT THE TOWN ENGINEERING OFFICES, TOWN D.P.W., AND UTILITY COMPANIES. OTHER UNDERGROUND UTILITIES MAY EXIST. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO VERIFY THE LOCATION, SIZE & ELEVATION OF ALL UTILITIES WITHIN THE AREA OF PROPOSED WORK AND TO CONTACT "DIG-SAFE" AT 811 AT LEAST 72 HOURS PRIOR TO ANY EXCAVATION, DEMOLITION OR CONSTRUCTION.

3) EVIDENCE OF UNDERGROUND IRRIGATION SYSTEM HAVE BEEN OBSERVED AT IRRIGATION HANDHOLES. LOCATION OF SAID LINES ARE UNKNOWN.

4) EVIDENCE OF UNDERGROUND ELECTRIC LINES HAVE BEEN OBSERVED AT ELECTRIC SOCKETS AND LAWN LIGHTS. CONNECTIONS OF SAID LINES ARE UNKNOWN.

5) ZONING SET-BACK LINES SHOWN HEREON PER HART ASSOCIATES BASED ON DISCUSSION WITH CAMBRIDGE BUILDING DEPARTMENT.

<u>LEGEND</u>

	SURFACE CONTOUR EDGE OF PAVEMENT CHAIN LINK FENCE WOOD FENCE CURB WITH TOP AND BOTTOM CURB FLEVATION
	EDGE OF VEGETATION
<u> </u>	SEWERLINE & MANHOLE WITH PIPE SIZE, MATERIAL & FLOW DIRECTION
1 <u>2"RCP</u>	DRAINLINE WITH PIPE SIZE, MATERIAL & FLOW DIRECTION, CATCHBASIN, MANHOLE & ROUND CATCHBASIN
₩ <u>6"C/</u>	WATER MANHOLE, WATER MAIN TEE, GATE VALVE & FIRE HYDRANT
G ^V <i>G10″DI</i>	GAS MAIN WITH SIZE & GATE VALVE
0HW-	EXISTING UTILITY POLE WITH DESIGNATION OVERHEAD WIRES AND GUY POLE
Ē	ELECTRIC MANHOLE & UNDERGROUND ELECTRIC LINES
///////////////	EDGE OF LAWN
	ZONING SET-BACK LINE
	HEDGE LINE
× 55.5	SPOT ELEVATION
5 4 3 9 3. 7 "" 12"	PROMINENT DECIDUOUS TREE WITH ELEVATION, SIZE
₩¥ 96.2 ************************************	PROMINENT CONIFEROUS TREE WITH ELEVATION, SIZE
¢	LAWN LIGHT
RCP	REINFORCED CONCRETE PIPE
SMH (S)	SEWER MANHOLE
DMH (D)	DRAIN MANHOLE
CB ⊞	CATCH BASIN
$\Box GM$	GAS METER
$\Box EM$	ELECTRIC METER
	CONNECTION UNKNOWN
0	FUST SICN
(R) (ED)	
	RON ROD
I.ROD U	VERTICAL CRANITE CURR
RIT CONC	RITUMINOUS CONCRETE
CLF	CHAINI LINK FENCE
<u>امع</u>	ROOF DRAIN
	HVAC UNIT
ς /M	STONE AND MASONRY
GRAN	GRANITE
IRR.	IRRIGATION HANDHOIF

	ELEVATION BENCH MARKS DATUM: CAMBRIDGE CITY BASE (CCB)	
NO.	DESCRIPTION	ELEV.
1.	U POLE 6 – SPIKE 2' A.G.	56.81
2.	OHW POLE - X-CUT ON N.W. CORNER	49.40
3.		
	SCALE: 1" = 10	9

10

RECORD LOCATION OF MANHOLE SEEMING

12 LAKEVIEW AVENUE

Cambridge, Massachusetts 02138

PREPARED FOR:



50 Church Street Belmont, Massachusetts 02478



Civil Engineers

Land Surveyors

Wetland Scientists

185 CENTRE STREET, DANVERS, MA 01923 VOICE (978) 777–3050, FAX (978) 774–7816 WWW.HANCOCKASSOCIATES.COM



EXISTING CONDITIONS PLAN OF LAND IN CAMBRIDGE, MA





REFERENCES: DEED BOOK 71824, PAGE 24 PLAN 1136 OF 1965

RECORD OWNER: JEFFERSON M & ELIZABETH GREEN CASE

ZONING: RESIDENCE A-1

<u>NOTES:</u>

1) THIS PLAN HAS BEEN PREPARED TO SHOW PROPOSED CHANGES TO EXISTING DWELLING AND APPURTENANCES.

2) ELEVATIONS SHOWN HEREON REFER TO THE CITY OF CAMBRIDGE BASE. PROJECT BENCH MARK IS AN "X" CUT IN OHW POLE ON NORTHWEST CORNER, ON THE NORTH SIDE OF STEP IN POLE, AT THE INTERSECTION OF LAKEVIEW AND BRATTLE. ELEVATION IS 49.40 (CCB).

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<u>LEGEND</u>

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	С	LF	
	S,	/M	
	GR.	AN.	
	W.	W.	
	Т.Е	3. <i>R</i> .	

EDGE OF PAVEMENT CHAIN LINK FENCE WOOD FENCE CURB LINE ZONING SET-BACK LINE LAWN LIGHT
POST
SIGN
RECORD
FOUND
IRON ROD
VERTICAL GRANITE CURB
BITUMINOUS CONCRETE
CHAIN LINK FENCE
STONE AND MASONRY
GRANITE PROPOSED WINDOW WELL
IC DE MENOVED



SCALE: 1" = 10'

20

10

OPEN SPACE CALCULATIONS: (AS PROVIDED BY GREGORY LOMBARDI DESIGN)				
EXISTING PERMEABLE AREA: 18,733± S.F.				
PROPOSED PERMEABLE AREA:	18,518± S.F.			
REQUIRED PERMEABLE AREA:	15,118± S.F.			

 \cap

Cambridge, Massachusetts 02138

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DATE: 6/10/2019 DRAWN BY: AAF/JMS SCALE: 1"=10' CHECK BY: JMS



PLOT DATE: Jun 10, 2019 4:09 pm PATH: F:\Civil 3D Projects\214	71 — Hart — Cambridge\DWG\
DWG: 21471EC.dwg	
LAYOUT: PP12 (2)	

SHEET: 1 OF 1

PROJECT NO .:

40

21471

	<u>LEGEND</u>		
<u>EXISTING</u>		<u>PROPOSED</u>	
54	SURFACE CONTOUR		
X	EDGE OF PAVEMENT FENCE		
S	SEWERLINE DRAINLINE WITH CLEANOUT	M	-1
	ROOF DRAIN WITH DOWNSPOUT	€RD	56.7 ×
W	WATER LINE	——— w ———— w ———	16"
G	GAS MAIN	6	مر المحقق 55.6 mm
	DRAIN MANHOLE		WOOD
	STRAW WATTLE BARRIER		55.8 55.8
<u>PLAN INTENT</u>	TO BE REMOVED		
THE PURPOSE OF THESE PLANS IS	5 TO SHOW THE PROPOSED STORMWAT	ER DRAINAGE SYSTEM AND UTILITY SERVICE	4.7*
<u>GENERAL NOTES</u>			55.8
1. CONTRACTOR SHALL FURNISH CON PROFESSIONAL LAND SURVEYOR. I	ISTRUCTION LAYOUT OF BUILDING AND SITE PROPERTY LINES SHOWN HERON ARE APPR	IMPROVEMENTS. THIS WORK SHALL BE PERFORMED	BY A
2. SAFETY MEASURES, CONSTRUCTION	N METHODS AND CONTROL OF WORK SHALL	L BE RESPONSIBILITY OF CONTRACTOR.	
3. CONTRACTOR SHALL BE RESPONSI	IBLE FOR REPAIR AND/OR REPLACEMENT O	F ANY EXISTING IMPROVEMENTS DAMAGED DURING AL HEREON DAMAGED IMPROVEMENTS SHALL BE	
REPAIRED TO THE SATISFACTION	OF THEIR RESPECTIVE OWNERS.		54.8 55.5
4. THIS PLAN IS NOT INTENDED TO S ELEVATIONS OF FOOTINGS, WALLS STRUCTURAL DRAWINGS	SHOW AN ENGINEERED BUILDING FOUNDATIO AND SUBSURFACE DRAINAGE TO PREVENT	N DESIGN, WHICH WOULD INCLUDE DETAILS AND FIN. INTERIOR FLOODING. SEE ARCHITECTURAL AND/OR	
5. ANY INTENDED REVISION OF THE	HORIZONTAL AND/OR VERTICAL LOCATION	OF IMPROVEMENTS TO BE CONSTRUCTED AS SHOWN	54.1
HEREON SHALL BE REVIEWED AND	APPROVED BY ENGINEER PRIOR TO IMPLE		
6. KIM ELEVATIONS SHOWN FOR NEW TAKEOFFS. FINISH RIM ELEVATIONS OTHERWISE	SIRUCIURES ARE APPROXIMATE AND ARE SSHOULD MATCH PAVEMENT, GRADING OR	PROVIDED TO ASSIST CONTRACTOR WITH MATERIAL LANDSCAPING, UNLESS SPECIFICALLY INDICATED	
7. WHERE EXISTING UTILITY LINES/ST	RUCTURES ARE TO BE CUT/BROKEN DOWN	ABANDONED, LINES/STRUCTURES SHALL BE	· · · · · · · · · · · · · · ·
PLUGGED/CAPPED/FILLED IN ACCO	ORDANCE WITH CITY REQUIREMENTS.		
6. THE CONTRACTOR SHALL VERIFY T CONSTRUCTION. ANY DISCREPANCT	ITTE LUCATION AND KELATIVE ELEVATION OF Y SHALL BE REPORTED TO THE ENGINEER.	- DEIVON MARKS PRIUR IU CUMMENCEMENT OF	N. OR STONE WALK
9. STRUCTURE DETAILS FROM INDEPE VERIFY THAT DETAILS SHOWN MAT	ENDENT VENDORS ARE CONSTANTLY CHANG TCH CURRENT DETAILS AND SPECIFICATIONS	NING. PRIOR TO CONSTRUCTION, THE CONTRACTOR SHEET S	
10. PROPOSED BUILDING FOUNDATION AS TO CONFORMANCE WITH FINAL	CONFIGURATION AND LOCATION ON THE LO	OT AS SHOWN ARE CONCEPTUAL AND SHALL BE VER	RIFIED
11. SITE LAYOUT AND GRADING DESIG	W IS BY LANDSCAPE ARCHITECT AND IS SI	HOWN HEREIN FOR REFERENCE ONLY.	
12. THE ENGINEER SHALL INSPECT THI	E BOTTOM OF EXCAVATION AT THE CHAMB	ER SYSTEM PRIOR TO INSTALLATION.	54.0
13. PIPE SLOPES SHOWN ARE IN FOO	Т/F00Т.		53.57 26.7
REGULATORY NOTES		0 4T 4 999 744 7077 4T LEAST 70 LOUDS DOWN	DD RET. 51.5
COMMENCEMENT OF ANY WORK.	-SAFE FOR UNDERGROUND UTILITY MARKIN	G AT T-888-344-7233 AT LEAST 72 HOURS PRIUM	
2. CONTRACTOR SHALL MAKE HIMSELI PERMITS AND APPROVALS ISSUED	F AWARE OF ALL CONSTRUCTION REQUIREM BY REGULATORY AUTHORITIES PRIOR TO C	IENTS, CONDITIONS, AND LIMITATIONS IMPOSED BY OMMENCEMENT OF ANY WORK. CONTRACTOR SHALL	6' WO
2. CONTRACTOR SHALL MAKE HIMSELI PERMITS AND APPROVALS ISSUED COORDINATE AND OBTAIN ALL CON 3. ALL WORK OUTSIDE OF BUILDING D	F AWARE OF ALL CONSTRUCTION REQUIREM BY REGULATORY AUTHORITIES PRIOR TO C ISTRUCTION PERMITS REQUIRED BY REGULA THAT IS LESS THAN 10 FEET FROM THE INS	IENTS, CONDITIONS, AND LIMITATIONS IMPOSED BY OMMENCEMENT OF ANY WORK. CONTRACTOR SHALL TORY AUTHORITIES. SIDE FACE OF BUILDING FOUNDATIONS SHALL CONFO	PATIO
 CONTRACTOR SHALL MAKE HIMSELI PERMITS AND APPROVALS ISSUED COORDINATE AND OBTAIN ALL CON ALL WORK OUTSIDE OF BUILDING T WITH THE UNIFORM STATE PLUMBIN 	F AWARE OF ALL CONSTRUCTION REQUIREM BY REGULATORY AUTHORITIES PRIOR TO C ISTRUCTION PERMITS REQUIRED BY REGULA THAT IS LESS THAN 10 FEET FROM THE INS NG CODE OF MASSACHUSETTS, 248 CMR 2.	IENTS, CONDITIONS, AND LIMITATIONS IMPOSED BY OMMENCEMENT OF ANY WORK. CONTRACTOR SHALL TORY AUTHORITIES. SIDE FACE OF BUILDING FOUNDATIONS SHALL CONFO 00.	PATIO
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 CONTRACTOR SHALL MAKE HIMSELI PERMITS AND APPROVALS ISSUED COORDINATE AND OBTAIN ALL CON ALL WORK OUTSIDE OF BUILDING T WITH THE UNIFORM STATE PLUMBIN ALL CONSTRUCTION SHALL CONFOR SOIL TEST DATA: <u>TP-1</u> 0-4" TOP SOIL, SANDY LOAM 4-18" FILL, LOAMY SAND 18-24" ORGANIC, CLAY LOAM 24-60" C1, CLAY 	F AWARE OF ALL CONSTRUCTION REQUIREM BY REGULATORY AUTHORITIES PRIOR TO C ISTRUCTION PERMITS REQUIRED BY REGULA THAT IS LESS THAN 10 FEET FROM THE INS NG CODE OF MASSACHUSETTS, 248 CMR 2. RM TO THE CITY OF CAMBRIDGE DPW STAN THE CITY OF CAMBRIDGE DPW STAN 0-4" TOP SOIL, SANDY LOAM 4-30" FILL, LOAMY SAND 30-114" C1, CLAY LOAM 114"+ C2, CLAY CPOLUDE ELEVATION-54 70	IENTS, CONDITIONS, AND LIMITATIONS IMPOSED BY COMMENCEMENT OF ANY WORK. CONTRACTOR SHALL TORY AUTHORITIES. SIDE FACE OF BUILDING FOUNDATIONS SHALL CONFO 00.	PRM SLATE PATIO 51.49 51.65 51.6
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 2. CONTRACTOR SHALL MAKE HIMSELI PERMITS AND APPROVALS ISSUED COORDINATE AND OBTAIN ALL CON 3. ALL WORK OUTSIDE OF BUILDING T WITH THE UNIFORM STATE PLUMBIN 4. ALL CONSTRUCTION SHALL CONFOR <u>SOIL TEST DATA:</u> <u>TP-1</u> 0-4" TOP SOIL, SANDY LOAM 4-18" FILL, LOAMY SAND 18-24" ORGANIC, CLAY LOAM 24-60" C1, CLAY 60-112" C2, CLAY LOAM 112"+ C3, CLAY GROUND EL.=57.25 REDOX AT 112" NO WEEPING, NO STANDING WATER <u>TP-2</u> 0-4" TOP SOIL, SANDY LOAM 	F AWARE OF ALL CONSTRUCTION REQUIREM BY REGULATORY AUTHORITIES PRIOR TO C ISTRUCTION PERMITS REQUIRED BY REGULA THAT IS LESS THAN 10 FEET FROM THE INS NG CODE OF MASSACHUSETTS, 248 CMR 2. RM TO THE CITY OF CAMBRIDGE DPW STAN 4-30" FILL, LOAMY SAND 30-114" C1, CLAY LOAM 114"+ C2, CLAY GROUND ELEVATION=54.70 REDOX AT 112" NO WEEPING, NO STANDING WATER IP-4 WATER LINES ENCOUNTERED TEST PIT ABANDONED TP-5	NENTS, CONDITIONS, AND LIMITATIONS IMPOSED BY COMMENCEMENT OF ANY WORK. CONTRACTOR SHALL TORY AUTHORITIES. SIDE FACE OF BUILDING FOUNDATIONS SHALL CONFO 00. IDARDS.	$RM = \frac{51.49}{51.49} = \frac{51.49}{51.49} = \frac{51.49}{51.6} = \frac{51.6}{51.6} = 5$
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 CONTRACTOR SHALL MAKE HIMSELI PERMITS AND APPROVALS ISSUED COORDINATE AND OBTAIN ALL CON- 3. ALL WORK OUTSIDE OF BUILDING T WITH THE UNIFORM STATE PLUMBIN ALL CONSTRUCTION SHALL CONFOR SOIL TEST DATA: IP-1 0-4" TOP SOIL, SANDY LOAM 4-18" FILL, LOAMY SAND 18-24" ORGANIC, CLAY LOAM 24-60" CT, CLAY 60-112" C2, CLAY LOAM 112"+ C3, CLAY GROUND EL.=57.25 REDOX AT 112" NO WEEPING, NO STANDING WATER IP-2 0-4" TOP SOIL, SANDY LOAM 4-30" FILL/ASH, LOAMY SAND 30-122" C HORIZON, CLAY LOAM GROUND ELEVATION=56.00 NO WEEPING, NO STANDING WATER, NO REDOX TEST PITS OBSERVED BY JACOB T. LEME OF HANCOCK ASSOCIATES ON FEBRUARY 	F AWARE OF ALL CONSTRUCTION REQUIREM BY REGULATORY AUTHORITIES PRIOR TO C ISTRUCTION PERMITS REQUIRED BY REGULA THAT IS LESS THAN 10 FEET FROM THE INS IG CODE OF MASSACHUSETTS, 248 CMR 2. RM TO THE CITY OF CAMBRIDGE DPW STAN 19–30" FILL, LOAMY SAND 30–114" C1, CLAY LOAM 114"+ C2, CLAY GROUND ELEVATION=54.70 REDOX AT 112" NO WEEPING, NO STANDING WATER 19–4 TOP SOIL, SANDY LOAM 4–19" FILL, LOAMY SAND 19–38" FILL/ASH, LOAMY SAND 38–113" CLAY LOAM, CLAY LOAM GROUND EL.=57.30' EXX (SE14144) '7, 2019.	NENTS, CONDITIONS, AND LIMITATIONS IMPOSED BY COMMENCEMENT OF ANY WORK. CONTRACTOR SHALL TORY AUTHORITIES. SIDE FACE OF BUILDING FOUNDATIONS SHALL CONFO 00. IDARDS. 50.77 109.07 49.80 109.07 49.80 109.07 49.80 109.07 49.33 109.07 109	PATIO = PATI
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 2. CONTRACTOR SHALL MAKE HIMSELI PERMITS AND APPROVALS ISSUED COORDINATE AND OBTAIN ALL CONS. 3. ALL WORK OUTSIDE OF BUILDING TO WITH THE UNIFORM STATE PLUMBIN. 4. ALL CONSTRUCTION SHALL CONFORSION SHALL CONFORSION SHALL CONFORMATION STATE PLUMBIN. 4. ALL CONSTRUCTION SHALL CONFORSION SHALL CONFORSION SHALL CONFORMATION SHALL CONFORMATION STATE PLUMBIN. 4. ALL CONSTRUCTION SHALL CONFORSION SHALL CONFORMATION SHALL CONFORMATION SHALL CONFORMATION SHALL CONFORMATION STAND SOLUTION CONFERMENT OF THE STATE OF SOLL SANDY LOAM GROUND ELEVATION STANDING WATER THE STATE OF FULL, CAMP STANDING WATER OF HANCOCK ASSOCIATES ON FEBRUARY ELEVATION BEN DATUM: CAMBRIDGE CONFERMENT. 	F AWARE OF ALL CONSTRUCTION REQUIREM BY REGULATORY AUTHORITIES PRIOR TO C ISTRUCTION PERMITS REQUIRED BY REGULA THAT IS LESS THAN 10 FEET FROM THE INS VG CODE OF MASSACHUSETTS, 248 CMR 2. RM TO THE CITY OF CAMBRIDGE DPW STAN TP=3 0-4" TOP SOIL, SANDY LOAM 4-30" FILL, LOAMY SAND 30-114" C1, CLAY LOAM 114"+ C2, CLAY ROUND ELEVATION=54.70 REDOX AT 112" NO WEEPING, NO STANDING WATER TP=4 WATER LINES ENCOUNTERED TEST PIT ABANDONED TP=5 0-4" TOP SOIL, SANDY LOAM 4-19" FILL, LOAMY SAND 39-113" CLAY LOAMY SAND 39-113" CLAY LOAMY SAND 39-113" CLAY LOAMY SAND 39-113" CLAY LOAMY SAND SGOUND EL.=57.30' CW (SE141144) '7, 2019.	$\begin{array}{c} \text{WITS, CONDITIONS, AND LIMITATIONS IMPOSED BY OMMENCEMENT OF ANY WORK. CONTRACTOR SHALL STORY AUTHORITIES. SIDE FACE OF BUILDING FOUNDATIONS SHALL CONFO. SIDE FACE OF BUILDING FOUNDATIONS SHALL CONFO. 00. DARDS. \begin{array}{c} \text{SO.7} \\ $	$\begin{array}{c c c c c c c c c c c c c c c c c c c $
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	B No. 7822 CAMBRIDGE
	A R C H I T E C T S Hart Associates, Inc. phone 617-489-0030 fax 617-489-0091 50 Church Street Belmont, Massachusetts 02478
	12 Lakeview Avenue Cambridge, MA
	EXISTING BASEMENT FLOOR PLAN scale: 1/4"=1'-0"
	ISSUED 1 06.06.2019 PERMIT SET 2 06.16.2021 REISSUED 3 4 1 4 1 1 5 1 1 6 1 1
ET 06.06.2019	EX1.0 06-16-2021

PERMIT SET 06.06.2019



	No. 7822 CAMBRIDGE
	A R C H I T E C T S Hart Associates, Inc. phone 617-489-0030 fax 617-489-0091 50 Church Street Belmont, Massachusetts 02478
	12 Lakeview Avenue Cambridge, MA
	EXISTING FIRST FLOOR PLAN scale: 1/4"=1'-0"
	ISSUED 1 06.06.2019 PERMIT SET 2 06.16.2021 REISSUED 3 4 1 5 1 1 6 1 1
ET 06.06.2019	EX1.1



	BRIDGE CAMBRIDGE
	A R C H I T E C T S Hart Associates, Inc. phone 617-489-0030 fax 617-489-0091 50 Church Street Belmont, Massachusetts 02478
	12 Lakeview Avenue Cambridge, MA
	EXISTING SECOND FLOOR PLAN scale: 1/4"=1'-0"
	ISSUED 1 06.06.2019 PERMIT SET 2 06.16.2021 REISSUED 3 4 1 5 6 1 6 1 1
T 06.06.2019	EX1.2 06.16.2021



	B No. 7822 CAMBRIDGE
	A R C H I T E C T S Hart Associates, Inc. phone 617-489-0030 fax 617-489-0091 50 Church Street Belmont, Massachusetts 02478
	12 Lakeview Avenue Cambridge, MA
	EXISTING THIRD FLOOR PLAN scale: 1/4"=1'-0"
	ISSUED 1 06.06.2019 PERMIT SET 2 06.16.2021 REISSUED 3 1 1 4 1 1 5 1 1 6 1 1
PERMIT SET 06.06.2019	EX1.3
REISSUED (06.16.2021



2 CARRIAGE HOUSE ATTIC PLAN SCALE: 1/4" = 1'-0"



















DEMOLIT SCALE: 1/4"	$\frac{\text{ION } \text{Ba}}{= 1' - 0''}$	ASEM	EN	T	FLO
	EXISTING	WALLS	BE	REM	IOVED
	EXISTING	WALLS	ТО	REM	IAIN

1. ALL WORK SHALL CONFORM TO STATE AND LOCAL CODES AND THE REQUIREMENTS OF THE LOCAL FIRE DEPARTMENT.

3. THE GENERAL CONTRACTOR SHALL KEEP THE PROJECT GENERALLY CLEAN OF ALL DEBRIS AND PICK UP AT THE END OF EACH WORKDAY. 4. ALL WORK SHALL BE DONE IN A WORKMANLIKE MANNER. MATERIALS AND EQUIPMENT TO COMPLY WITH AND BE INSTALLED ACCORDING TO

5. THE CONTRACTOR(S) SHALL FAMILIARIZE WITH HIM/HERSELF WITH AND VERIFY EXISTING SITE CONDITIONS AND EXISTING STRUCTURE. ADVISE

6. THE CONTRACTOR SHALL REVIEW WITH THE OWNER THE EXTENT OF THE DEMOLITION PRIOR TO COMMENCEMENT OF THE WORK. SPECIAL CONSIDERATION SHOULD BE GIVEN TO PROTECT AND SEGREGATE AREAS NOT SCHEDULED FOR MODIFICATIONS. 7. CAREFULLY PROTECT ALL FINISH SURFACES TO REMAIN. ALL EXISTING EXTERIOR SURFACES ON EXISTING WALLS TO REMAIN AND BE

	No. 7822 CAMBRIDGE
PARTMENT. THE END OF EACH WORKDAY. ND BE INSTALLED ACCORDING TO ND EXISTING STRUCTURE. ADVISE MENT OF THE WORK. SPECIAL VALLS TO REMAIN AND BE	A R C H I T E C T S Hart Associates, Inc. phone 617-489-0030 fax 617-489-0091 50 Church Street Belmont, Massachusetts 02478
	12 Lakeview Avenue Cambridge, MA
	BASEMENT BASEMENT DEMOLITION PLAN scale: 1/4"=1'-0"
	ISSUED 1 06.06.2019 PERMIT SET 2 06.16.2021 REISSUED 3 4 1 4 1 1 5 1 1 6 1 1
PERMIT SET 06.06.2019	D1.0
REISSUED	06.16.2021

	B No. 7822 CAMBRIDGE
	A R C H I T E C T S Hart Associates, Inc. phone 617-489-0030 fax 617-489-0091 50 Church Street Belmont, Massachusetts 02478
	12 Lakeview Avenue Cambridge, MA
	FIRST FLOOR DEMOLITION scale: 1/4"=1'-0"
	ISSUED 1 04.15.2019 PERMIT SET 2 06.16.2021 REISSUED 3 4 1 4 1 1 5 1 1 6 1 1
ET 06.06.2019	D1.1 06-16 2021

(1)	DEMOLIT	$\frac{10N}{1}$ SI	ECON	D	FLOOR
\bigcirc	JUALE. 1/4	_ 1 = 0			
		EXISTING	WALLS	ΒE	REMOVED
		EXISTING	WALLS	ТО	REMAIN

R PLAN

	No. 7822 CAMBRIDGE
	A R C H I T E C T S Hart Associates, Inc. phone 617-489-0030 fax 617-489-0091 50 Church Street Belmont, Massachusetts 02478
	12 Lakeview Avenue Cambridge, MA
	SECOND FLOOR DEMOLITION PLAN scale: 1/4"=1'-0"
	ISSUED 1 06.06.2019 PERMIT SET 2 06.16.2021 REISSUED 3 4 1 5 1 1 6 1 1
ET 06.06.2019 REISSUED	D1.2 06.16.2021

PERMIT SET 06.06.2019

	No. 7822 CAMBRIDGE
	A R C H I T E C T S Hart Associates, Inc. phone 617-489-0030 fax 617-489-0091 50 Church Street Belmont, Massachusetts 02478
	12 Lakeview Avenue Cambridge, MA
	THIRD FLOOR DEMOLITION PLAN scale: 1/4"=1'-0"
	ISSUED 1 06.06.2019 PERMIT SET 2 06.16.2021 REISSUED 3 4 1 5 1 1 6 1 1
ET 06.06.2019 REISSUED	D1.3 06.16.2021

PERMIT SET 06.06.2019

	A R C H I T E C T S Hart Associates, Inc. phone 617-489-0030 fax 617-489-0091 50 Church Street Belmont, Massachusetts 02478
ROOF PEAK AT HIGHEST POINT EL: 93'-6"	12 Lakeview Avenue Cambridge, MA
COVERED PORCH ADDED 03.17.2022	SOUTH EXTERIOR ELEVATIONS scale: 1/4"=1'-0"
EL: 57'-1 ¹ 6"	ISSUED 1 06.06.2019 PERMIT SET 2 05.28.2021 PERMIT ADDENDUM 3 06.16.2021 REISSUED 4 03.17.2022 COVERED PORCH REVISIONS 5 0 OVERED PORCH REVISIONS 6 0 0
03.17.2022 UPDATED WITH REDUCED COVERED PORCH AND ADDED COVERED PORCH PERMIT SET 06.06.2019	A2.2

	A R C H I T E C T S A R C H I T E C T E C T E C T E C T E C T E C T E C T E C T E C T E C T
	12 Lakeview Avenue Cambridge, MA
	NORTH. EXTERIOR ELEVATIONS scale: 1/4"=1'-0"
	ISSUED 1 06.06.2019 PERMIT SET 2 05.28.2021 PERMIT ADDENDUM 3 06.16.2021 REISSUED 4 03.17.2022 COVERED PORCH REVISIONS 5 03.17.2022 COVERED PORCH REVISIONS 6 0 0
03.17.2022 UPDATED WITH REDUCED COVERED PORCH AND ADDED COVERED PORCH PERMIT SET 06.06.2019	A2.3

WINDOW SCHEDULE

Туре	Name	Width	Height	Style	Otv	Comments	Mull pocket
Турс	. (unite	frame dim	frame dim	Style	χij	Comments	in poener
MAIN HOUS	E	I I			1		1
Α	Not Used					2" Thick Subsill Defer to	
	Little Harbor					drawings for tempered	
В	Double Hung	3'-5 3/4"	6'-4 3/8"	Wood Frame Wood Sash Double Hung Window	13	window locations. One	
	Window			Double finding window		window used in C-B-C	
	2 Gang Little					Assembly in Music Room.	
B2	Harbor Double	7'-3 1/2" (2@3'-	6'-4 3/8"	Wood Frame Wood Sash	4 - two gang	drawings for tempered	4"
	Hung Window	5 3/4")		Double Hung Window		window locations.	
	Little Herber					2" Thick Subsill. Refer to	
С	Double Hung	2'-5 1/2"	6'-4 3/8"	Wood Frame Wood Sash	4	window locations. Two	7 3/8" Mull Pocket for
C	Window			Double Hung Window		window used in C-B-C	window C-B-C Assembly.
						Assembly in Music Room.	
n	Little Harbor	31 5 3/4"	<i>/</i> ! 0 1/8"	Wood Frame Wood Sash	2	2" Thick Subsill. Refer to	
D	Window	5-5 5/4	4-9 1/8	Double Hung Window	Σ	window locations.	
	2 Gang Little	7'-5 1/4" (2@3'-		Wood Frame Wood Sash		2" Thick Subsill. Refer to	
E2	Harbor Double	6 5/8")	6'-4 3/8"	Double Hung Window	4 - two gang	drawings for tempered	4"
	Little Harbor					2" Thick Subsill. Refer to	
F	Double Hung	3'-0"	5'-4 1/4"	Wood Frame Wood Sash	26	drawings for tempered	
	Window					window locations.	
F7	2 Gang Little Harbor Double	6'-4" (2@3'-0")	5'-4 1/4"	Wood Frame Wood Sash	1 - two gang	2" Thick Subsill. Refer to	<u></u> <i>1</i> "
1' <i>2</i>	Hung Window	(2@3-0)	5-4 1/4	Double Hung Window	1 - two gang	window locations.	7
	Little Harbor			Wood Frame Wood Sash		2" Thick Subsill. Refer to	
G	Double Hung	2'-9 1/2"	4'-8 1/2"	Double Hung Window	2	drawings for tempered	
	Little Harbor					2" Thick Subsill. Refer to	
Н	Double Hung	2'-10 1/2"	5'-2 3/4"	Wood Frame Wood Sash	2	drawings for tempered	
	Window					window locations.	
T	Little Harbor Double Hung	2'-9 1/2"	4'-2"	Wood Frame Wood Sash	7	2" Thick Subsill. Refer to drawings for tempered	
-	Window	2 9 172	. 2	Double Hung Window	,	window locations.	
	2 Gang Little	6'-2" (2@2'-		Wood Frame Wood Sash	_	2" Thick Subsill. Refer to	
J2	Harbor Double	11")	4'-11"	Double Hung Window	l - two gang	drawings for tempered	4"
	3 Gang Little	71.011.(2.001.4				2" Thick Subsill. Refer to	
K3	Harbor Casement	$7^{-2^{*}}(3@2^{-4})$	4'-4''	Casement Window	1 - three gang	drawings for tempered	Direct Mull (0")
	Window 2 Gang Little					window locations.	
L3	Harbor Casement	6'-4 1/2" (3@2'-	3'-9"	Wood Frame Wood Sash	1 - three gang	drawings for tempered	Direct Mull (0")
	Window	1 1/2")		Casement Window		window locations.	
М	Little Harbor	21.0.1/21	2 10"	Wood Frame Wood Sash	1	2" Thick Subsill. Refer to	
IVI	Window	2-9 1/2	3-10	Double Hung Window	1	window locations.	
	3 Gang Little	7'-1 1/2" (3@2'-		Wood Frame Wood Sash		2" Thick Subsill. Refer to	
N3	Harbor Casement	4 1/2")	3'-9"	Casement Window	1 - three gang	drawings for tempered	Direct Mull (0")
	2 Gang Marvin		[]]]	Clad Frame Clad Sash		2" Thick Subsill, Refer to	
02	Inswing Casement	5'-8 1/2" (2@2'-	{ <u>4'21/2</u> " }	Inswing Casement	3 - two gang	drawings for tempered	
CADDIACEL	Window	101/17	Current	Window		window locations.	
CANNIAGE I	Little Harbor			WestE W 10.1		2" Thick Subsill. Refer to	
Р	Double Hung	2'-9 1/2"	6'-0"	Wood Frame Wood Sash Double Hung Window	6	drawings for tempered	
0	Window Existing To Demai					window locations.	
R3	Existing To Remain						
NOTES:			1				
1.	All windows to have	E Low-E Insulating	g glass		Maril Tra	N. Entering C. 1	
2.	Screens at new double hung SDL windows to be half screens with Hi Transparency Mesh screens UON. Exterior screen frame color to be determined.						
3.	All frames and expo	sed wood to have	exterior and inte	rior finish of prime cost unle	ess otherwise noted	(UON) All window jamh lin	ers to be white UON
3. 4	4. All double hung window hardware finish to be determined. No sash lifts at double hung windows.						
5.	 Find double hang window hardware missile be determined. The sash miss at double hang windows. New SDL windows: All muntin bars to be fixed, 7/8" with internal spacer bars. Space bar to be Bronze. Provide historic sill profile at all windows UON. Sills to extend beyond edge of casing 1" UON Typical exterior casings except basement windows to be 1"x 5" (actual dimension) flat UON with backband, clear pre-primed western red cedar, no finger joints typ. at all windows. Exterior casings to be primed and factory installed, UON. 						
6							
7							
8	• Manufacturer to prov	vide shop drawing	gs for approval pr	rior to ordering windows.			
9	Sash 1 3/4" thick.						
10	Refer to plans A1.0,	A1.1, A1.2, A1.3	, and A1.4 and e	xterior elevations A2.1, A2.2	for additional info	ormation.	
11	All Windows to be N	Mahogany					
12	Window sticking pro	ofile to be determi	ned				

DOOR SCHEDULE

*DO NOT
N

INDOW & 12 Lakeview Avenue SCHEUDLE Hart Associates, Inc. Cambridge, MA Ender 617-489-0030 ALE: NTS 50 Church Street					
INDOW & CHEUDLE 12 Lakeview Avenue Cambridge, MA					
INDOW & SCHEUDLE Ale: NTS					
WINDOW & DOOR SCHEUDLE scale: NTS					
ISSUED 1 06.06.2019 PERMIT SET 2 06.16.2021 REISSUED 3 4 1 5 1 1 6 1 1					
A2.5					

REISSUED

	BILLEN A. THERE DARCHITCH
	A R C H I T E C T S Hart Associates, Inc. phone 617-489-0030 fax 617-489-0091 50 Church Street Belmont, Massachusetts
	12 Lakeview Avenue Cambridge, MA
	BUILDING SECTIONS scale: 1/2"=1'-0"
	ISSUED 1 06.06.2019 PERMIT SET 2 06.16.2021 REISSUED 3 4 1 5 6 1 6 1 1
PERMIT SET 06.06.2019 REISSUED	A3.1 06.16.2021

		A R C H I T E C T S A R C H I T E C T S Hart Associates, Inc. phone 617-489-0030 fax 617-489-0031 fax 617-489-00000 fax 617-480-0000000000000000000000000000000000
2: ROOF CONSTRUCTION T SEAM COPPER ROOF NTINUOUS LAYER ICE AND WATER SHIL 3" CDX PLYWOOD SHEATHING TERS – REFER TO STRUCTURAL AWINGS FOR SIZE AND SPACING DETA 49 ICYNENE CLOSED CELL SPRAY YURETHANE INSULATION 12 1	ELD ILS	12 Lakeview Avenue Cambridge, MA
	_ TYP. NEW WALL CONSTRUCTION CEDAR CLAPBOARDS TO MATCH EXITING "HOME SLICKER" "BLUESKIN VP" 2" CDX PLYWOOD SHEATHING 2X6 WOOD STUDS @ 16" O.C. R-19 ICYNENE CLOSED CELL SPRAY POLYURETHANE FOAM INSULATION CERTAINTEED "MEMBRAIN" VAPOR RETARDER 8" BLUEBOARD W/ 8" SMOOTH SKIM COAT PLASTER SMOOTH FINISH ALL WALLS	BUILDING SECTIONS scale: 1/2"=1'-0"
		ISSUED 1 06.06.2019 PERMIT SET 2 06.16.2021 REISSUED 3 4 4 6 6 6 6 6 6 6 6 6 7 6 7 7 7 7 7 7 7
	PERMIT SET 06.06.2019	
	REISSUED	06.16.2021

3 LIFE SAFETY THIRD FLOOR PLAN SCALE: 1/8" = 1'-0"

ELECTRICAL LEGEND					
SYMBOL	DESCRIPTION	REMARKS			
S	SMOKE DETECTOR	PROVIDE AS REQUIRED BY CODE. EXACT MODEL TO BE DETERMINED.			
H	HEAT DETECTOR	PROVIDE AS REQUIRED BY CODE. EXACT MODEL TO BE DETERMINED.			
\$ CO	COMBINATION SMOKE AND CARBON MONOXIDE DETECTOR	PROVIDE AS REQUIRED BY CODE. EXACT MODEL TO BE DETERMINED.			

	REMARKS	_		
DETECTOR	PROVIDE AS REQUIRED BY CODE.			
DETECTOR	PROVIDE AS REQUIRED BY CODE. EXACT MODEL TO BE DETERMINED.			
NATION SMOKE AND CARBON	PROVIDE AS REQUIRED BY CODE. EXACT MODEL TO BE DETERMINED.		A R C H I T E C T S Hart Associates, Inc.	50 Church Street Belmont, Massachusetts 02478
			12 Lakeview Avenue Cambridge, MA	
			LIFE SAFETY PLANS	SCALE: $1/4"=1'-0"$
		ISSIIED	1 06.06.2019 PERMIT SET 2 03.19.2021 LIFE SAFETY PLAN ADDED 3 03.26.2021 LIFE SAFETY PLAN UPDATED 4 06.16.2021 REISSUED	
PERMIT	SET 06.06.20	19	A5.1	
	REISSU	ED 06	6.16-2	021

GENERAL CONDITIONS

- 1. ALL WORK SHALL COMPLY WITH THE REQUIREMENTS OF THE COMMONWEALTH OF MASSACHUSETTS STATE RESIDENTIAL CODE NINTH EDITION.
- 2. G.C. MUST BUILD EXACTLY WHAT IS SHOWN ON STRUCTURAL DRAWINGS. ANY PROPOSED DEPARTURES FROM WHAT IS INDICATED MUST BE REVIEWED WITH THE ENGINEER PRIOR TO CONSTRUCTION. ALL UNAUTHORIZED CHANGES TO THE APPROVED DRAWINGS MUST BE REMOVED AND REPLACED AT THE CONTRACTOR'S EXPENSE.
- 3. ENGINEER'S DESIGN IS DERIVED FROM ASSUMED FIELD CONDITIONS. ANY DISCREPANCIES BETWEEN WHAT IS SHOWN ON OUR DOCUMENTS AND WHAT IS FOUND IN THE FIELD MAY CHANGE THE STRUCTURAL DESIGN, AND MUST IMMEDIATELY BE BROUGHT TO THE ENGINEER'S ATTENTION PRIOR TO ANY CONSTRUCTION.
- THE CONTRACTOR SHALL CAREFULLY VERIFY ALL DIMENSIONS AND CONDITIONS SHOWN ON DRAWINGS PRIOR TO COMMENCEMENT OF THE WORK, AND SHALL NOTIFY THE ENGINEER IMMEDIATELY OF ANY DISCREPANCIES BETWEEN ENGINEERING AND ARCHITECTURAL DOCUMENTS.
- THE CONTRACTOR IS RESPONSIBLE FOR ALL MEANS AND METHODS OF TEMPORARY SHORING, BRACING, OR OTHERWISE PROTECTING ANY PORTION OF THE STRUCTURE, SITE AND UTILITIES FROM DAMAGE DURING CONSTRUCTION. THE ENGINEER IS SPECIFYING THE FINISHED CONDITION ONLY, WITHOUT ASSUMING KNOWLEDGE NOR DECONDIDUCTION THE CONTRACTOR WITH ADJUSTED THEOREM.
- RESPONSIBILITY FOR HOW THE CONTRACTOR WILL ACHIEVE THIS RESULT.
 6. FOR EXACT LOCATIONS OF FLOOR AND ROOF OPENINGS, POSTS, ETC., SEE ARCHITECTURAL DRAWINGS.

FOUNDATIONS

- 1. EXCAVATE TO LINES AND GRADES REQUIRED TO PROPERLY INSTALL THE FOUNDATIONS ON INORGANIC, UNDISTURBED SOIL OR CONTROLLED STRUCTURAL BACKFILL AS REQUIRED BY THE ARCHITECT. ALL EXCAVATIONS SHALL BE DRY BEFORE PLACING ANY CONCRETE.
- 2. EXTERIOR FOOTINGS SHALL BE PLACED ON APPROVED SOIL AT A MINIMUM DEPTH OF 4 FEET, OR AS MODIFIED BY THE STRUCTURAL ENGINEER, BELOW THE LOWEST ADJACENT GROUND EXPOSED TO FREEZING. ANY ADJUSTMENT OF FOOTING ELEVATIONS DUE TO FIELD CONDITIONS MUST HAVE THE APPROVAL OF THE ARCHITECT.
- 3. SOIL BEARING CAPACITY: FOOTINGS MUST BE PLACED ON SOIL WITH A MINIMUM BEARING CAPACITY OF 4000 POUNDS PER SQUARE FOOT.
- BACKFILL BELOW FOOTINGS AND SLABS SHALL BE MADE WITH APPROVED GRANULAR MATERIALS PLACED IN 6" LAYERS. LAYERS SHALL BE COMPACTED TO 96% DENSITY AT OPTIMUM MOISTURE CONTENT, AS DEFINED BY ASTM D1557, METHOD D.
- BACKFILLING AGAINST WALLS OR PIERS MAY ONLY BE DONE AFTER WALLS OR PIERS ARE BRACED TO PREVENT MOVEMENT. FOR WOOD FRAMED RESIDENTIAL CONSTRUCTION, NO BACKFILLING OF WALLS MAY TAKE PLACE UNTIL THE FIRST FLOOR DECK HAS BEEN FRAMED AND SHEATHED, UNLESS WRITTEN APPROVAL IS GIVEN BY THE ARCHITECT OR ENGINEER.
- 6. PROVIDE FOUNDATION DRAINAGE, WATERPROOFING/DAMP-PROOFING, AND FOUNDATION WALL INSULATION AS INDICATED ON THE ARCHITECTURAL DRAWINGS.
- 7. PROVIDE METAL OR PVC SLEEVES IN THE FOUNDATION WALLS FOR SEWER, GAS, ELECTRIC, AND WATER LINES, AS REQUIRED.

<u>CONCRETE</u>

- ALL CONCRETE WORK SHALL BE PERFORMED IN CONFORMANCE WITH THE LATEST EDITION OF ACI-318, "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE".
 CONCRETE SHALL ACHIEVE A MINIMUM 28 DAY DESIGN STRENGTH AS FOLLOWS:
- FOOTINGS, WALLS, INTERIOR SLABS-ON-GRADE, AND OTHER CONCRETE NOT OTHERWISE SPECIFIED - 3000 PSI. EXTERIOR SLABS EXPOSED TO WEATHER - 4000 PSI.
 SLUMP AT THE POINT OF DISCHARGE FROM THE READY-MIX TRUCK SHALL BE 3-5".
- REINFORCING STEEL: TYPICAL ASTM A615, GRADE 60. FIELD BENT ASTM A615, GRADE 40
 WELDED WIRE FABRIC - ASTM A185.
- WELDED WIRE FABRIC ASTMATOS.
 NON-SHRINK GROUT SHALL BE "EMBECO 153" BY MASTER BUILDERS, "SONOGROUT" BY SONNEBORN BUILDING PRODUCTS, "FIVE STAR GROUT" BY U.S. GROUT CORPORATION, OR EQUAL AS APPROVED BY THE OWNER.

STRUCTURAL STEEL

- 1. STRUCTURAL STEEL WORK SHALL CONFORM TO THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION: "SPECIFICATION FOR STRUCTURAL STEEL FOR BUILDINGS", LATEST EDITION
- STEEL BEAMS SHALL CONFORM TO ASTM A992, WITH A MINIMUM YIELD STRENGTH OF 50 KSI.
- 3. PLATES, ANGLES, CHANNELS, AND MISC. FABRICATED HARDWARE SHALL CONFORM TO ASTM A36, WITH A MINIMUM YIELD STRENGTH OF 36 KSI. RECTANGULAR STEEL TUBING
- SHALL CONFORM TO ASTM A500, GRADE B, WITH A MINIMUM YIELD STRENGTH OF 46 KSI.
 ALL STEEL TO STEEL FIELD CONNECTIONS SHALL BE MADE BY HIGH STRENGTH BOLTING WITH ASTM A325 BOLTS OR WELDING WITH E70 XX ELECTRODES. STEEL TO CONCRETE AND STEEL TO WOOD FIELD CONNECTIONS MAY BE MADE WITH ASTM A 307 BOLTS.
- 5. STEEL SHALL BE SHOP-PAINTED WITH A MODIFIED ALKYD PRIMER UNLESS OTHERWISE
- NOTED. 6. ALL STRUCTURAL STEEL EXPOSED TO THE WEATHER SHALL BE GALVANIZED. SEE
- ARCHITECTURAL DRAWINGS AND SPECIFICATIONS FOR PAINTING REQUIREMENTS.
 STRUCTURAL STEEL SHOP DRAWINGS SHALL BE PREPARED AND SUBMITTED TO THE ARCHITECT FOR APPROVAL. THESE DRAWINGS SHALL SHOW COMPLETE AND ACCURATE MEMBER LAYOUT, SIZES, GRADE, DIMENSIONS, CONNECTIONS, OPENINGS, ACCESSORIES, AND ALL OTHER INFORMATION NECESSARY FOR COMPLETE AND ACCURATE FABRICATION AND ASSEMBLY OF THE MEMBERS. PROVIDE TEMPLATES OR LOCATIONS DRAWINGS FOR INSTALLATION OF ANCHOR BOLTS. A SUBMITTAL SHALL BE BY PDF.
- 8. NO CUTTING OF OR OPENINGS THROUGH STEEL WILL BE PERMITTED WITHOUT THE WRITTEN APPROVAL OF THE ENGINEER.

STEEL DECK

I OCATIONS.

- STEEL DECK SHALL CONFORM TO THE "STANDARD FOR COMPOSITE STEEL FLOOR DECK (ANSI/SDI-C1.0)" AND THE "SPECIFICATION FOR DESIGN OF LIGHT GAUGE COLD-FORMED STEEL STRUCTURAL MEMBERS (AISI)."
- STEEL DECK PANELS SHALL BE FORMED FROM STEEL SHEETS CONFORMING TO ASTM A653 STRUCTURAL QUALITY, WITH A MINIMUM YIELD POINT OF 50,000 PSI.
 STEEL ELOOD DECK SUMULIDE DYNUM ODAET, NUMBER OF PERSONNEL AND THE PROVINCE AND
- STEEL FLOOR DECK SHALL BE BY VULCRAFT, 2VLI18 COMPOSITE DECKING, 2" DEPTH x 18 GAGE GALVANIZED G60 COMPOSITE METAL DECK CONFORMING TO ASTM A653 (Fy = 50KSI).
 STEEL DECK CROSS SECTIONS ARE ONLY CEREFICIENT OF CONTRACT OF CONTRACT. CONTRACT OF CO
- STEEL DECK CROSS SECTIONS ARE ONLY REPRESENTED DIAGRAMMATICALLY ON THE DRAWINGS.
 FASTEN METAL DECK TO SUPPORTING STRUCTURE W/ MIN 5/8" PUDDLE WELDS IN A 36/4
- PATTERN FOR 2VLI DECK AND A 36/3 PATTERN FOR 1.5VLI DECK.
 6. WHERE DECK SPAN IS GREATER THAN 5 FEET, FASTEN SIDE LAPS AND PERIMETER EDGES W/#10 SELE DPULLING SOPEWIS @ 21/0" O O DPOLIDE DATE. AT DEDITIETED EDGES
- EDGES W/#10 SELF DRILLING SCREWS @ 3'-0" O.C. PROVDE PAFs AT PERIMETER EDGES SUPPORTED BY CONCRETE.
 7. CONCRETE SHALL BE HAVE A MINIMUM COMPRESSIVE STRENGTH OF 4000 PSI AT 28 DAYS. REINFORCE THE SLAB WITH 6x6 W1.4xW1.4 WWF SUPPORTED ON CHAIRS AT 3'-0" CENTERS. REFER TO PLAN FOR LIGHTWEIGHT AND NORMAL WEIGHT SLAB

ROUGH CARPENTRY

- ALL STRUCTURAL ROUGH CARPENTRY SHALL CONFORM WITH THE "NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION," ANSI/AWS NDS-2015.
 UNLESS NOTED OTHERWISE, STRUCTURAL WOOD FRAMING SHALL HAVE THE
- FOLLOWING MINIMUM PROPERTIES:
- A. FOR 2-4" (NOMINAL) THICKNESS KILN DRIED MEMBERS: SPRUCE-PINE-FIR (SPF) NO.1/NO.2 OR BETTER WITH THE FOLLOWING PROPERTIES:
 - a. ALLOWABLE BENDING STRESS, Fb = 875 PSI
 b. ALLOWABLE SHEAR STRESS, Fv = 135 PSI
 - COMPRESSION PARALLEL TO GRAIN = 1,150 PSI
 - d. COMPRESSION PERPENDICULAR TO GRAIN = 425 PSI
 e. MODULUS OF ELASTICITY, E = 1,400,000 PSI
- B. FOR 2-4" (NOMINAL) THICKNESS PRESSURE TREATED (PT) MEMBERS:
- SOUTHERN PINE NO.2 OR BETTER WITH THE FOLLOWING PROPERTIES: a. ALLOWABLE BENDING STRESS, Fb = 750 PSI
- ALLOWABLE SHEAR STRESS, Fv = 175 PSI
- c. COMPRESSION PARALLEL TO GRAIN = 1,250 PSI d. COMPRESSION PERPENDICULAR TO GRAIN = 565 PSI
- e. MODULUS OF ELASTICITY, E = 1,400,000 PSI
- C. FOR 5" (NOMINAL) THICKNESS AND LARGER KILN DRIED AND PRESSURE TREATED (PT) MEMBERS: SOUTHERN PINE NO.1 OR BETTER WITH THE FOLLOWING PROPERTIES:
 - a. ALLOWABLE BENDING STRESS, Fb = 1,350 PSI
 - b. ALLOWABLE SHEAR STRESS, Fv = 165 PSI c. COMPRESSION PARALLEL TO GRAIN = 825 PSI
 - d. COMPRESSION PARALLEL TO GRAIN = 623 PSI d. COMPRESSION PERPENDICULAR TO GRAIN = 375 PSI
- e. MODULUS OF ELASTICITY, E = 1,500,000 PSI

3. ENGINEERED LUMBER PRODUCTS SHALL BE TRUSJOIST BY WEYERHAEUSER AS A MINIMUM STANDARD OF QUALITY. SUBSTITUTIONS ARE NOT ALLOWED UNLESS SPECIFICALLY APPROVED BY THE ENGINEER. ENGINEERED LUMBER FRAMING SHALL HAVE THE FOLLOWING MINIMUM PROPERTIES:

- A. LAMINATED VENEER LUMBER (LVL):
 - a. ALLOWABLE BENDING STRESS, Fb = 2,600 PSI
 b. ALLOWABLE SHEAR STRESS, Fv = 285 PSI
 - c. COMPRESSION PARALLEL TO GRAIN = 2,510 PSI
 - d. COMPRESSION PERPENDICULAR TO GRAIN = 750 PSI e. MODULUS OF ELASTICITY, E = 2,000,000 PSI
- B. PARALLEL STRAND LUMBER COLUMNS
 - a. ALLOWABLE BENDING STRESS, Fb = 2,400 PSI
 b. ALLOWABLE SHEAR STRESS, Fv = 190 PSI
 - c. COMPRESSION PARALLEL TO GRAIN = 2,500 PSI
 - d. COMPRESSION PERPENDICULAR TO GRAIN = 545 PSI
- e. MODULUS OF ELASTICITY, E = 1,800,000 PSI
- ENGINEERED LUMBER JOISTS SHALL BE "TJI" BY TRUS JOIST BY WEYERHAEUSER. JOIST SERIES AND SIZE SHALL BE AS INDICATED ON THE CONSTRUCTION DRAWINGS. INSTALL JOISTS IN CONFORMANCE WITH MANUFACTURER'S REQUIREMENTS.
- UNLESS SUPPORTED BY HANGERS, ALL JOISTS SHALL HAVE A MINIMUM END BEARING OF 2" AND A MINIMUM INTERMEDIATE BEARING OF 3-1/2".
 ALL STRUCTURAL SHEATHING SHALL CONFORM TO PS 1, PS 2 AND RE ARA RATED WITH
- 6. ALL STRUCTURAL SHEATHING SHALL CONFORM TO PS-1, PS-2 AND BE APA RATED WITH THE MINIMUM PROPERTIES: A. SUBFLOOR SHEATHING SHALL BE MINIMUM 23/32" THICK TONGUE AND GROOVE,
- EXPOSURE 1, C-D GRADE PLYWOOD OR STRUCTURAL 1 GRADE ADVANTECH OSB SHEATHING. PANELS SHALL HAVE A MINIMUM 24" SPAN RATING. FLOOR SHEATHING SHALL BE GLUED TO FLOOR JOISTS WITH AN APPROVED ADHESIVE PRIOR TO NAILING.
- B. ROOF SHEATHING SHALL BE MINIMUM 19/32" THICK, EXPOSURE 1, C-D GRADE PLYWOOD OR STRUCTURAL 1 GRADE HUBER ZIP SYSTEM ROOF SHEATHING PANELS. PANELS SHALL HAVE A MINIMUM 40/20 SPAN RATING. FOR SPANS OF 24" AND GREATER, PROVIDE TONGUE AND GROOVE EDGES OR METAL H-CLIPS CENTERED BETWEEN RAFTERS.
- C. WALL SHEATHING SHALL BE MINIMUM 15/32" THICK, EXPOSURE 1, C-D GRADE PLYWOOD OR 7/16" THICK STRUCTURAL 1 GRADE HUBER ZIP SYSTEM SHEATHING PANELS. PANELS SHALL HAVE A MINIMUM 24/16 SPAN RATING.
- 7. UNLESS INDICATED OTHERWISE, ALL ENGINEERED LUMBER JOISTS SHALL BE INSTALLED IN A SIMPLE SPAN CONFIGURATION.
- ALL WOOD HAVING DIRECT CONTACT WITH CONCRETE OR MASONRY, AND WHEREVER WOOD IS WITHIN 8" OF FINISHED GRADE OR PART OF OPEN DECK CONSTRUCTION, SHALL BE PRESSURE TREATED.
- SHALL BE PRESSURE TREATED.
 NON-PT BEAM ENDS SET IN CONCRETE BEAM POCKETS SHALL BE WRAPPED IN A SELF-ADHERING RUBBER MEMBRANE.
- 10. HANGERS AND CONNECTORS SHALL BE BY SIMPSON STRONG-TIE CORP. THE CONTRACTOR SHALL STRICTLY ADHERE TO MANUFACTURER'S FASTENING REQUIREMENTS. ALL CONNECTORS SHALL BE INSTALLED WITH THE MAXIMUM FASTENER QUANTITY, UNLESS NOTED OTHERWISE. ALL CONNECTORS EXPOSED TO WEATHER SHALL BE ZMAX GALVANIZED COATED.
- UNLESS NOTED OTHERWISE, MULTIPLE 2x POSTS, SUCH AS 2-2x6, 3-2x6, ETC., SHALL BE CONFIGURED WITH ONE 2x AS A KING STUD AND THE BALANCE AS JACK STUDS.
- 12. SOLID LUMBER POSTS WITHIN WALLS SHALL BE INSTALLED IN JACK CONFIGURATION WITH ONE ADJACENT 2x KING STUD ATTACHED TO POST.
- 13. UNLESS NOTED OTHERWISE, PROVIDE AT LEAST TWO JACK STUDS BENEATH ENDS OF 2X12 AND ENGINEERED LUMBER HEADERS AND BEAMS.
- 14. FOR WOOD JOIST SPANS UP TO 14 FEET, PROVIDE A SINGLE ROW OF FULL DEPTH BLOCKING BETWEEN JOISTS AT MID SPAN. FOR SPANS EXCEEDING 14 FEET, PROVIDE TWO ROWS OF FULL DEPTH BLOCKING BETWEEN JOISTS AT THIRD POINTS OF THE SPAN
- PROVIDE SOLID BLOCKING BETWEEN JOISTS AT ALL JOIST BEARING LOCATIONS.
 PROVIDE DOUBLE JOIST UNDER PARALLEL PARTITION WALLS OR SOLID BLOCKING @ 32" O.C. BETWEEN THE CLOSEST TWO JOISTS.
- GABLE-END WALL STUDS IN CATHEDRAL, PARTIAL CATHEDRAL, OR HIGH CEILING SPACES SHALL SPAN UNINTERRUPTED FROM THE FLOOR PLATE TO THE UNDERSIDE OF THE ROOF RAFTERS. THEY SHOULD NOT BE INTERRUPTED BY ANY HORIZONTAL PLATES OR BEAMS, UNLESS NOTED OTHERWISE ON THE DRAWINGS.
- MEMBERS WITHIN BUILT-UP BEAMS, WHETHER MADE OF SAWN OR ENGINEERED LUMBER, SHALL ONLY BE SPLICED OVER SUPPORTS.
- 19. PROVIDE SIMPSON H2.5A HURRICANE TIES BETWEEN EACH RAFTER BOTTOM AND ITS BEARING POINT.
- 20. UNLESS ANOTHER CONNECTOR IS CALLED OUT, PROVIDE ONE SIMPSON A35 FRAMING ANCHOR AT EACH RAFTER/RIDGE BEAM INTERSECTION, AND TWO WHEN RAFTERS ARE DOUBLED OR TRIPLED (ONE EACH SIDE).
- CONTRACTOR SHALL CAREFULLY COORDINATE THE WORK OF ALL TRADES TO MINIMIZE THE NEED FOR CUTS AND BORE HOLES IN FRAMING LUMBER. IN GIRDERS, BEAMS, OR JOISTS, CUTS AND BORE HOLES SHALL NOT BE DEEPER THAN 1/5 THE MEMBER DEPTH NOR MORE THAT 2" IN DIAMETER, AND SHALL NOT BE LOCATED NEARER TO THE END OF THE SPAN THAN THREE TIMES THE MEMBER DEPTH NOR WITHIN THE CENTER THIRD OF THE SPAN UNLESS REINFORCED TO MEET STRESS CALCULATIONS.
- 22. AT WOOD POSTS LANDING ON FLOOR DECK, PROVIDE SOLID <u>VERTICAL</u> WOOD BLOCKING WITHIN DECK SANDWICH TO LINK UPPER POST WITH LOWER SUPPORT. BLOCKING TO MATCH UPPER POST SIZE.
- 23. SET LVL BEAMS THAT FRAME FLUSH WITH DIMENSIONAL LUMBER JOISTS 1/4" BELOW THE TOP OF JOISTS TO ALLOW FOR JOIST SHRINKAGE. WHERE BEARING WALLS OR POSTS LAND ON THESE BEAMS, INFILL GAP WITH 1/4" PLYWOOD FOR SOLID BEARING.

DESIGN LOADS PER 2015 INTERNATIONAL RESIDENTIAL CODE

GROUND SNOW LOAD [780 CMR 51.0 TABLE R301.2(4)]

LIVE LOADS	
ATTICS WITHOUT STORAGE:	10 PSF
ATTICS WITH LIMITED STORAGE:	20 PSF
HABITABLE ATTICS, ATTICS SERVED W	ITH FIXED STAIRS
AND SLEEPING AREAS:	30 PSF
EXTERIOR BALCONIES AND DECKS	40 PSF
ALL OTHER AREAS	40 PSF
WIND LOAD [780 CMR 51.0 TABLE R301.2(4)]	128 MPH, EXPOSURE B
<u>DEAD LOAD</u>	WEIGHTS OF MATERIALS AND CONSTRUCTION

40 PSF

MAXIMUM SOLAR PANEL DEAD LOAD 5 PSF

PER RB103.5 SOLAR-READY ZONES OF ROOF ARE INDICATED ON THE ROOF FRAMING PLAN.

POST LEGEND

SUPPORT UP ID	2007 UD
20	POSTUP
CU	COLUMN UP
_C UP	LALLY COLUMN UP
SUPPORT DOWN ID	
}-2x4	(3) 2x4 POST
3-2x6	(3) 2x6 POST
12-24	(2) 2x4 JACK STUD + (1) 2x4 KING STUD
P44	4x4 FIR POST
P66	6x6 FIR POST
PT 4x4	4x4 ACQ PRESSURE TREATED POST
PT 6x6	6x6 ACQ PRESSURE TREATED POST
HSS 31/2	3 ¹ / ₂ x3 ¹ / ₂ x ¹ / ₄ HOLLOW STRUCTURAL SECTION
_C 3 ¹ / ₂	3 ¹ / ₂ "Ø CONCRETE FILLED LALLY COLUMN
HDU4	SIMPSON HDU4-SDS2.5 HOLDOWN
HDU5	SIMPSON HDU5-SDS2.5 HOLDOWN
PSL 31/2x31/2	3 ¹ / ₂ x3 ¹ / ₂ PARALLEL STRAND LUMBER POST
PSL 31/2x51/4	3 ¹ / ₂ x5 ¹ / ₄ PARALLEL STRAND LUMBER POST
PSL 31/2x7	3 ¹ / ₂ x7 PARALLEL STRAND LUMBER POST
PSL 51/4x51/4	5¼x5¼ PARALLEL STRAND LUMBER POST
PSL 5¼x7	5¼x7 PARALLEL STRAND LUMBER POST
PSL 7x7	7x7 PARALLEL STRAND LUMBER POST

DENOTES POST UP POST GRAPHICAL SYMBOL SYMBOL BUPOPTING

L DENOTES SUPPORTING POST

SYMBOLS LEGEND:

1.	SHEARWALL	=	
2.	INTERIOR BEARING WALL	=	
З.	BEARING WALL ABOVE	=	
4.	STEEL MOMENT RESISTING CONNECTION	=	

ABBREVIATION LEGEND

W/ WITH	A.B. ARCH. BM. BRG. BTWN. CLG. COL. CONC. CONC. CONT. CU (E) E.W. FDN. FTG. HDR. HORIZ. G.C. LC LVL MAX. MFR MIN. N.T.S. O.C. PT PU REQ. SPEC. TYP. U.N.O. VERT. V.O.P. V.I.F.	ANCHOR BOLT ARCHITECT BEAM BEARING BETWEEN CEILING COLUMN CONCRETE CONTINUOUS COLUMN UP EXISTING EACH WAY FOUNDATION FOOTING HEADER HORIZONTAL GENERAL CONTRACTOR LALLY COLUMN LAMINATED VENEER LUMBER MAXIMUM MANUFACTURER MINIMUM NOT TO SCALE ON CENTER PRESSURE TREATED POST UP REQUIRED SPECIFICATION TYPICAL UNLESS NOTED OTHERWISE VERTICAL VERIFY OR PROVIDE VERIFY IN FIELD
	V.I.F. W/	VERIFY IN FIELD WITH

SHEARWALL NOTES

1. SHEARWALLS CONSTRUCTION:

- SHEATHING TO BE ¹/₂" APA RATED STRUCTURAL SHEATHING, REFER TO PLAN AND SECTIONS FOR ADDITIONAL INFORMATION
- SHEATHING TO BE ATTACHED TO THE WALL STUDS WITH 8d NAILS
- BE PLAN CALLOUTS
 HOLDOWNS TO BE HDU BY SIMPSON. SEE PLAN FOR MODEL
- NUMBER AND THREADED ROD SIZE. 2. ALL PLYWOOD SEAMS IN A SHEARWALL SHALL BE BLOCKED WITH

DIMENSIONAL LUMBER OF THE SAME SIZE AS THE WALL STUDS.

3. REFER TO PLANS AND SECTIONS FOR STUD SIZES, STUDS SHALL BE SPACED AT 16 INCHES ON CENTER UNLESS NOTED OTHERWISE ON PLAN.

4. CARE SHOULD BE TAKEN TO ADJUST NAIL GUN PRESSURE SO AS TO NOT OVER DRIVE NAILS INTO PLYWOOD. NAIL HEADS SHOULD BE FLUSH WITH PLYWOOD FACE. OVER DRIVING NAILS GREATLY REDUCES THE EFFECTIVENESS OF THE SHEARWALL.

PLYWOOD SHEATHING

HOLDOWN UNIT SCHEDULE

			-			
SAI NOTATION	SIMPSON NAME	ANCHOR BOLT Ø	A.B. CONCRETE EMBED.			
DTT1Z DTT2Z HDU2 HDU4 HDU5 HDU8 HDU11 HDU14	DTT1Z DTT2Z-SDS2.5 HDU2-SDS2.5 HDU4-SDS2.5 HDU5-SDS2.5 HDU8-SDS2.5 HDU11-SDS2.5 HDU14-SDS2.5	1/2" 1/2" 5/8" 5/8" 5/8" 7/8" 1" 1"	6" ** 10" ** 18" * 18" * 18" * 18" * 24" *			
*MINIMUM ANCHOR BOLT CONCRETE EMBEDMENT VALUES FOR ALL HDU PRODUCTS BASED ON CAST-IN-PLACE CONSTRUCTION UNLESS NOTED OTHERWISE BY ENGINEER. **MINIMUM ANCHOR BOLT CONCRETE EMBEDMENT VALUE USING POST-INSTALLED ANCHOR BOLT SET WITH HILTI HIT-HY 200 ADHESIVE						

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12 Lakeview Avenue Cambridge, MA
GENERAL NOTES scale: NTS
IISSUED 1 06.06.2019 PERMIT SET 2 06.16.2021 REISSUED 3 4 1 4 1 1 5 1 1
S0.01

REISSUED

STEVEN PAUL SIEGEL STRUCTURAL

REISSUED 06.16.2021

Scale: 1/4"=1'-0"

THIRD FLOOR FRAMING PLAN

NOTES: ALL I-JOISTS TO BE SHEATHED ON TOP FLANGE W/ 1¹/₈" THICK PLYWOOD FLOOR SHEATHING.
 ALL I-JOISTS TO HAVE MID-SPAN BRIDGING.

3. ALL HEADERS IN EXTERIOR WALLS TO BE 3-2x6, TYP. U.N.O.

SHEAR WALL ON INSIDE FACE OF THIS ENTIRE WALL LENGTH, NO HOLDOWNS. PROVIDE 6" O.C. EDGE NAILING. -

PAUL STRUCTURAL

ROOF FRAMING PLAN

<u>NOTES:</u> 1. ALL HEADERS IN EXTERIOR WALLS ARE EXISTING, TYP. U.N.O.

Scale: 1/4"=1'-0"

402 SQ. FT.	EXISTING CARRIAGE HOUSE VOLUME TOTAL EXISTING VOLUME TO REMAIN 18,844 CU. FT.		No. 7822 CAMBRIDGE
	EXISTING HOUSE VOLUME TOTAL EXISTING VOLUME TO REMAIN 99,238 CU. FT. TOTAL EXISTING VOLUME TO BE REM 7,395 CU. FT. TOTAL EXISTING VOLUME HOUSE 106,663 CU. FT. EXISTING VOLUME CALCULATIONS #1 (Existing) #2 (Removing) #3 (Removing) #4 (Removing)	MOVED Cubic Feet 78300 286 4851 856 579	A R C H I T E C T S Hart Associates, Inc. phone 617-489-0030 fax 617-489-0091 50 Church Street Belmont, Massachusetts 02478
174 SQ. FT.	#6 (Removing) #7 (Existing) #7 (Existing) #8 (Existing) #9 (Existing) #10 (Existing) #11 (Existing) #12 (Existing) #13 (Existing) #14 (Removing) Main House Total Carriage House GRAND TOTAL EXISTING VOLUME 10% of Existing Volume Existing Area in Volume To Be Demolished Percentage of Existing Volume to be Demolished Demolition Permit Required for 25% or more of the	440 2934 8667 6913 1314 710 200 200 383 106633 18844 125477 12548 7395 5.89%	riew Avenue bridge, MA
#2 — REFER TO WEST ELEVATION AND PLAN FOR NUMBERS USED IN VOLUME CALCULATION #3 — REFER TO WEST ELEVATION AND PLAN FOR NUMBERS USED IN VOLUME CALCULATION	PROPOSED HOUSE VOLUME TOTAL EXISTING VOLUME TO REMAIN 99,238 CU. FT.		12 Lakev Camb
	TOTAL NEW VOLUME 19,415 CU. FT. TOTAL PROPOSED VOLUME HOUSE 118,653 CU. FT. PROPOSED VOLUME CALCULATIONS #1 (Existing) #1 (Existing) #7 (Existing) #8 (Existing) #10 (Existing) #11 (Existing) #12 (Existing) #15 (Adding)	Cubic Feet 78300 2934 8667 6913 1314 710 200 200 7513	EXISTING & PROPOSED VOLUME scale: 1/16"=1'-0"
#10 – 174 SQ. FT. ORTH GABLE - 34 SQ. FT. DORMER - #20 & 21	#13 (Adding) #16 (Adding) #17 (Adding) #17 (Adding) #18 (Adding) #19 (Adding) #20 (Adding) #20 (Adding) #21 (Adding) #22 (Adding) #22 (Adding) #23 (Adding) #23 (Adding) #24 (Adding) #25 (Adding) #26 (Adding) #26 (Adding) Main House Total Carriage House (same as existing) GRAND TOTAL PROPOSED VOLUME Volume Added (137,880 Cu.Ft125,477 Cu.Ft.) Volume Added (137,880 Cu.Ft125,477 Cu.Ft.) Volume Allowed as per 8.22.1.f (10% of the existing nonconforming structure) Percent Volume Added	575 2875 2425 4188 295 352 135 652 135 135 135 135 135 135 13844 137497 12020 12548 9.58%	ISSUED OS.28.2021 PERMIT ADDENDUM 2 06.16.2021 REISSUED 3 06.16.2021 REISSUED 4 0 0 5 0 0 6 0 0
PE	125,477 CU. FT. TOTAL PROPOSED VOLUME HOUSE & CARRIAGE 137,497 CU. FT. 9.58% TOTAL VOLUME ADDED (10% ALLOWED AS PER 8.22.1.f) CRMIT ADDENDUM 05.28	E HOUSE	PA-2

Lakeview Exist	ting Mean Grade						Senter Senter
A	Length of Wall Grade at sta	56.94	Grade at End Average Grade 57.2 57.07	Length*AverageGrade 1267.5247			STATISTICS AND
B	11.43 1.39	57.2 56.93	56.93 57.065 57.05 56.99	652.25295 79.2161			No. 7822
D	10.99	57.05	57.78 57.415	630.99085			CAMBRIDGE VETA
E	9.46 18.39	57.78 57.28	57.28 57.53 57.28 57.28	3 544.2338 3 1053.3792			A MARSON AND SAN AND AND AND AND AND AND AND AND AND A
G	29.27	57.28	57.36 57.32	2 1677.7564 1792.4436			Og manual.
	I 39.875	57.50	56.99 57.045	<u>i 1792.4430</u> <u>i 2274.669375</u>			
J	1.9 (18.45	56.99 56.99	56.99 56.99 57.12 57.055	108.281 1052.66475			
L	1.78	57.12	57.43 57.275	5 101.9495			
M N	1 18.48 1 40.9	57.43 57.19	57.19 57.31 56.17 56.68	1059.0888 2318.212			tts 0 0
P	13.14	56.17	58.09 57.13	8 750.6882			-003 -0091
Q	l 31.46	58.09	50.94 57.515	1810.5722			E (
tal Perimeter	300.465		Total Average Grade	e 17173.92343			I T Inc 617 617 617 at, M
	Average Crede Total (Total						elmone CS, Chu
	Average Grade Total Average Grade/Total						
	Perimeter) 57.1 Existing Roof Peak	93 5					A SSOC
		5515					
	Total Existing Building Height 36.3	21832					Iart
							ew Avenue idge, MA
- 2 Lakeview Pror	posed Mean Grade		<u>NORTH ELEVATION</u>				.akevi Cambri
/all	Length of Wall Grade at sta	rt	Grade at End Average Grade	Length*AverageGrade			
<i>A</i>	A 22.21 B 30.89	57 57.32	57.32 57.10 57.5 57.4	6 1269.5236 1 1773.3949			
(46.73	57.5	57.5 57.	5 2686.975			
L	E 19.42	53.5 57.5	<u>53.5</u> 53.5 57.5 57.1	5 374.5 5 1116.65			
I	F 28.875	57.5	57.54 57.53	2 1660.89			
ŀ	1 2.67	57.54	<u> </u>	5 153.525			
	I 5	53.5 57 5	53.5 53.5	5 267.5 5 219.075			
ķ	< <u>2.04</u>	57.5	57.5 57.	5 117.3			
I	L 3.83 1 12.42	57.5 57.5	<u> </u>	5 220.225 5 714.15			EE E
N	N 3.83	57.5	57.5 57.	5 220.225			
F	0.26	57.6	57.6 57.	6 1403.320123 6 14.976			116 ["]
C	2 1.67 R 7.3125	57.6 57.6	<u> </u>	6 96.192 3 419.00625			
9	S 30.86	57	57 5	7 1759.02			
l	J 1.67	57.22	57.22 57.22	2 95.5574			
۱ ۷	1.67 1.67 9.98	57.22 57.5	57.5 57.30 57.5 57.31	5 95.7912 5 573.85			XII S
>	X 24.71	57.5	57.5 57.	5 1420.825			
2	Z 5.85	53.5 57.5	53.5 53.1 57.5 57.1	5 336.375			
AA BE	9.98 3 1.67	57.5 57.5	57.5 57.1 57.5 57.5	5 573.85 5 96.025			
	C 1.6 20.91	57.5 57	57.5 57. 57	5 92 7 1600 17			
E	E 11.25	57	57 57	7 641.25			
FI	+ <u>1.67</u>	57	57 5	/ 95.19			
otal Perimeter	375 4375		Total Average Grade	21461 69635			
				21701.03033			
	Average Grade Total (Total Average Grade/Total						
	Perimeter) 57.16	449835			PROPOSED GRADES SHOWN		
		53.5			SUPERSEDE GRADES SHOWN ON		
	Total Existing Building Height 36.33	550165			SHEET CI DATED 6/7/2019		
							SSUED 05.28.202
							PA-3
<u>l</u>			NORTH ELEVATION				
					PERMIT ADDENDUM	05.28.2021	
					R	FISSUED (06 16 2021

12 Lakeview Existing Mean Grade Grade at start Grade Mail Length of Wall Grade at start Grade B 11.43 57.2 C 1.39 56.93 D 10.99 57.05 E 9.46 57.78 F 18.39 57.28 G 29.27 57.28 G 29.27 57.36 H 31.32 57.36 I 39.875 57.1 J 1.9 56.99 K 18.45 56.99 L 1.78 57.12 M 18.48 57.43 N 40.9 57.19 P 13.14 56.17 Q 31.48 58.09 I Q 31.48 O 30.465 7.1578168 Perimeter 300.465 7.1578168 I Average Grade Total (Total Average Grade/Total Perimeter) 57.1578168 Existing Roof Peak	Ade at End Average Grade Length*AverageGrade 57.2 57.07 1267.5247 56.93 57.065 652.25295 57.05 56.99 79.2161 57.78 57.415 630.99085 57.78 57.415 630.99085 57.78 57.415 630.99085 57.78 57.53 544.2338 57.28 57.32 1053.3792 57.36 57.32 1677.7564 57.1 57.23 1792.4436 56.99 57.045 2274.669375 56.99 57.045 2274.669375 56.99 57.055 1052.66475 57.12 57.31 1059.0888 56.17 56.68 2318.212 58.09 57.13 750.6882 56.17 56.68 2318.212 58.09 57.515 1810.5722 56.94 57.515 1810.5722 56.94 57.515 1810.5722 56.94 57.515 1810.5722		A R C H I T E C T S A R C H I T E C T S Hart Associates, Inc. phone 617-489-0030 fax 617-489-0091 50 Church Street Belmont, Massachusetts 02478
Image: Constraint of Wall Grade at start Gr Mail Length of Wall Grade at start Gr A 22.21 57 B 30.89 57.32 C 46.73 57.5 D 7 53.5	$\frac{1}{1269.5236}$		12 Lakeview Avenue Cambridge, MA
E 19.42 57.5 G 7 57.54 H 2.67 57.5 I 5 53.5 J 3.81 57.5 K 2.04 57.5 M 1.57.5 53.5 J 3.81 57.5 K 2.04 57.5 M 12.42 57.5 N 3.83 57.5 O 25.4375 57.5 P 0.26 57.6 Q 1.67 57.6 R 7.3125 57.6 S 30.86 57 U 1.67 57.22 W 9.98 57.5 M 9.98 57.5 M 7.3125 57 U 1.67 57.22 W 9.98 57.5 X 24.71 57.5 X 24.71 57.5 X 2.5.85 57.5<	57.557.51116.6557.5457.521660.8957.557.52402.6457.557.5153.52553.553.553.553.553.5219.07557.557.5219.07557.557.5219.07557.557.5219.07557.557.5220.22557.557.5220.22557.557.5220.22557.557.5220.22557.557.5220.22557.657.5220.22557.657.5220.22557.657.5220.22557.657.5220.22557.657.5220.22557.657.51463.92812557.657.51463.92812557.657.696.19257.757.3419.006255757.3419.006255757.357.357.257.357.557.557.557.857.557.557.857.557.557.857.557.5336.37557.557.557.857.557.557.857.557.557.857.557.557.857.557.557.857.557.557.857.557.557.857.557.557.857.557.557.857.557.557.857.557.557.		EXISTING & PROPOSED HEIGHT scale: 1/16"=1'-0"
CC 1.6 57.5 DD 29.81 57 EE 11.25 57 FF 1.67 57 Total Perimeter 375.4375 Average Grade Total (Total Average Grade/Total Perimeter) 57.16449835 Existing Roof Peak 93.5 Total Existing Building Height 36.33550165	57.5 57.5 57 57 57 57 57 57 57 57 57 57 57 57 95.19 7 57 57 95.19 7 57 57 95.19 7 57 57 95.19 7 57 7 57 7 57 95.19 7 7 57 7 57 7 57 7 57 7 57 7 57 7 57	PROPOSED GRADES SHOWN SUPERSEDE GRADES SHOWN ON SHEET C1 DATED 6/7/2019	ISSUED 1 05.28.2021 2 06.16.2021 3 06.16.2021 4 1 5 1 6 1
	NORTH ELEVATION	PERMIT ADDENDUM 05.28.2021	PA-3